100-YEAR FLOODPLAIN STUDY for an UNNAMED TRIBUTARY OF THE "MIDDLE RUN" in the "WILOMT MANOR - SECTION 8" SUBDIVISION

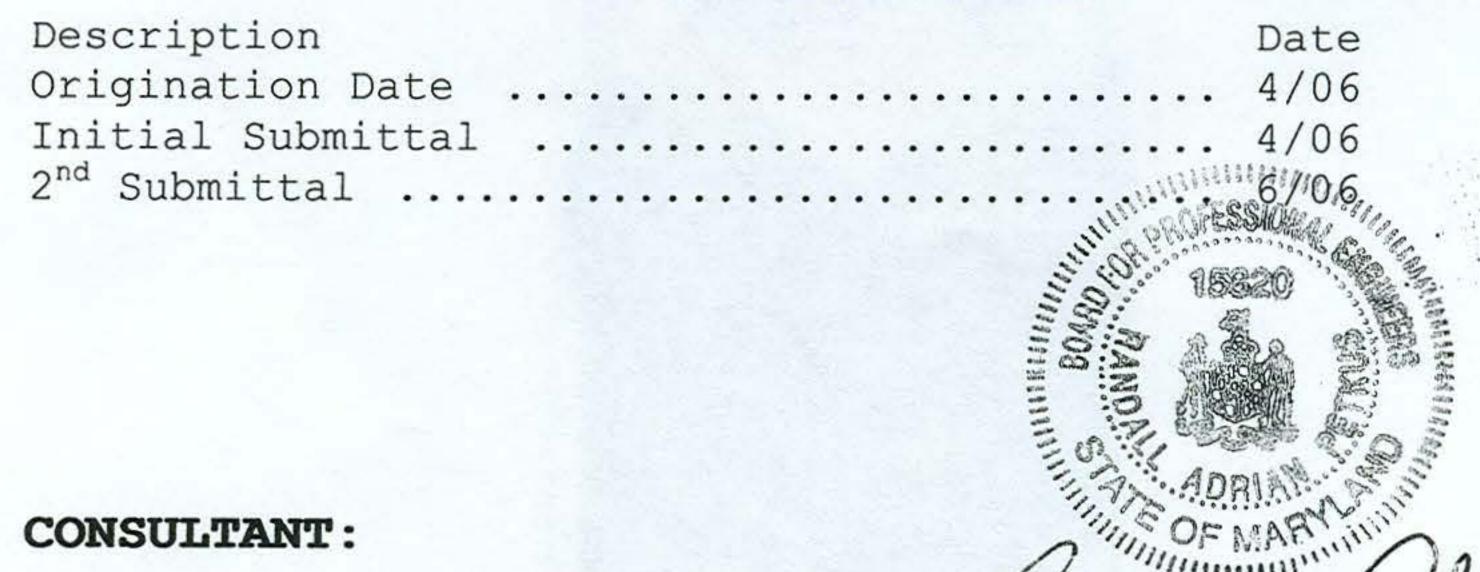
Carroll County File #P-05-014

INDEX OF SHEETS

Description	Pages
Title Sheet	1
Narrative	2-4
FEMA Panel Excerpt	5
TR55 Hydrology Computations (SA "A")	6-8
HY-8 Culvert Program Output	9-11
FLOWMASTER 2005 Computer Program Output	12-41
Drainage Area Map	42
USGS Map	43
Zoning Map Excerpt	44
Soils Map Excerpt	45
N-Factor Delineation Plan	46
100-Yr Floodplain Plan, Profile	Attachment 1
100-Yr Floodplain Cross-Sections	Attachment 2

SUBMITTAL DATES

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Pg 1/46

BPR, Inc. 150 Airport Drive, Unit #4 Westminster, Md. 21157

Handlan A tother Randall A. Petkus, P.E.

6/12/06

PRINTED(Rev.): 6/12/2006

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FLOODPLAIN STUDY NARRATIVE FOR WILMOT MANOR SECTION 8

C.C. File No. P-05-014

I BACKGROUND

PURPOSE

In accordance with Section 114-8A, B, "Delineations", of the Carroll County Floodplain Management Code, this study provides the computations to determine the Ultimate Condition 100-Yr. Flood Plain width for two Tributary's of Middle Run. It employs the "Simplified Method" as described in the Floodplain Management Manual to compute the hydraulic characteristics in representative stream channel sections obtained from topographic mapping and supplemental field measurements. The objective is to accurately compute the Flood Plain limits for the tributary as shown on the FIRM Panel, for the portion of the tributary that runs through Wilmot Manor Section 8. This is to document adherence to the applicable items of Section 114-7, "General Regulations" of the code. An approximate tributary location is shown on the FIRM on Panel #240015 0100 B, and designated as a "Zone C, Areas of minimal flooding" in the vicinity of the property.

EXISTING CONDITION

The property is located on the West side along Don Avenue, directly across from the intersection with East Court, in the 4th Election District, Carroll County, Md. It contains a total of 38.67 acres, and lies entirely within the "R-40" Zone. It is currently un-developed with one existing Barn and an out-building. Existing access to Don Avenue is provided by a stone driveway, coming off of a existing paved access at the intersection of Don avenue and Wilmot Ridge Road, located in the northern portion of the property. Groundcover is typical agricultural uses, being, crop fields bounded by small woods, tree rows and thick grass pastures.

The property location appears on FIRM Panel #240015 0100 B, and the portion of the tributary runs through the south portion of our property, with the discharge point at a culvert crossing under Don Avenue. The tributary runs east to west beginning at property line North 18 Degrees 00 Minutes 25 Seconds West 457.85 ft. and continuing easterly for approximately 1,250 ft., ending at the invert of a 36 inch RCP.

PROPOSED CONDITION

The project is proposing an 8 Lot Major Subdivision, and the total acreage is divided into typically 1.50 ac. lots, and one lot containing the remaining acreage of the total 38.67 ac. Subdivision. All proposed building areas lie within the Northern portion of the property, and will be developed with new single family residences. All residences will have access to Don Avenue provided by a proposed Cul-Da-Sac "South Bend Court", at the north end of the property. This proposed Cul-Da-Sac shall be conveyed to the County through a 44 ft. wide right-of-way and a 50 ft. wide right-of-way around the Cul-Da-Sac.

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Pg 2/46

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II METHODOLOGY

Hydrology

The 100-Yr. Stream Flow(Q) used to compute the 100-yr. floodplain Water Surface Elevation(WSE) was computed using the TR-55 method. The Drainage Area is comprised of several Aerial topography maps plotted at 5ft intervals as stated on the "DRAINAGE AREA MAP" enclosed and verified against USGS Map Westminster. The DA Map is overlaid with Carroll County Zoning Maps and Soils Conservation District Soils Maps, using CADD techniques and are listed on the DA Map. A Study Point location is chosen where the tributary discharges off the property through a 36in. RCP at the south easterly portion of he property, and is used as the downstream point to delineate the maximum contributory Drainage Area(DA), DA "A" = 155.2 ac., to any point along the tributary within the subdivision.

The Ultimate Condition Weighted Runoff Curve Number(CN) =56 and 100-Yr. runoff depth Q =2.28 in., is computed on the "TR-55 WORKSHEET 2, RUNOFF CURVE NUMBER AND RUNOFF" spreadsheet. To compute the weighted CN, the applicable composite RCN Cover Type is assigned to each Residential and Commercial Zoning Districts from the TR-55 manual, and the HGS areas are measured and tabulated separately on the spreadsheet. For the R-40 Zoning District, the RCN is derived by assigning the "1 ac. Lot" cover type to the maximum area of lot yield(1 lot/40000 Sq.ft.) permitted by zoning requirements. The resulting cover type breakdown is tabulated on part 1 of the spreadsheet.

Time of Concentration(Tc) is computed by standard TR55 methods on the "TR-55 WORKSHEET 3, TIME OF CONCENTRATION OR TRAVEL TIME" spreadsheet. The Tc path is shown and identified on the Drainage Area Map. Segment A-B is the 100 ft. maximum sheet flow length and occurs in the yards within the upstream properties of the residential zone. Segment B-C, C-D, and D-E is the limits of shallow concentrated flow throughout the Residential Zone. The remaining downstream segments E-F are defined channel lengths at different longitudinal slopes.

The ultimate condition 100-Yr. Storm flow =320.68 CFS is computed on the "TR-55 WORKSHEET 4, GRAPHICAL PEAK DISCHARGE METHOD" spreadsheet using the contributory area and hydrologic RCN and Tc data discussed above as input. The 2-Yr. storm flow is also computed as a guide to estimate bankfull flow and confirm the defined channel velocities for the Tc spreadsheet.

Hydraulics

Hydraulic Computations to determine the 100-Yr. Water Surface Elevations(W.S.E.) at 10 Crosssections(XS) are completed using the "FLOWMASTER 2005" computer program. The crosssections are specifically located to model abrupt changes in floodplain cross-section, horizontal & vertical alignments, and/or hydraulic structure interference. At a minimum, cross-sections are placed at each end of the study limits, and one in the middle. All XS channel geometry and other input used in the computations is printed as tabular form output and contained in this report. The maximum 100-Yr. flow computed at the downstream property line is used at all XS. A "N-FACTOR DELINEATION PLAN" using an Orthometric view of the existing site to show the different groundcover N-factor limits input for the left and right over-banks is provided in the report.

Pg 3/46

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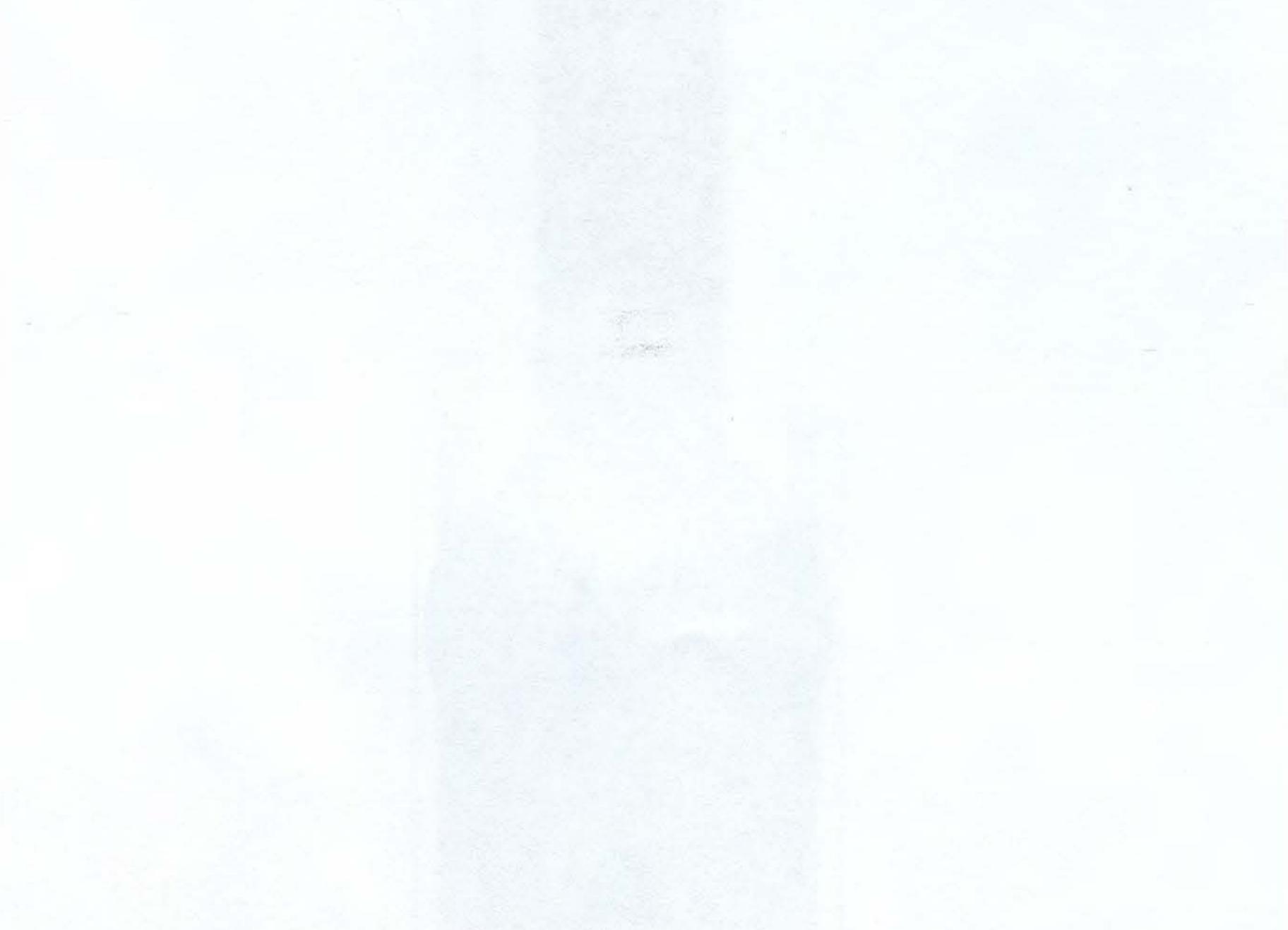
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Hydraulics (Continued)

Headwater and Tailwater elevations for Don Avenue culvert crossing is computed using the HY-8 Culvert Analysis computer program. The elevations are plotted at the culvert inlet and tailwater cross-section locations on the Water Surface Profile. The same 100 yr. flow(Q=320.68 cfs)determined for the study point is used for the culvert flow input for simplicity. A constant elevation of 713.50 was used to define the tail water. All roadway configuration and culvert alignment input data used in the analysis is shown on the "HY-8 Program Output" sheets contained in the report. The Headwater for the culvert overtops the roadway, and is conveyed downstream by weir flow. The WSE over Don Avenue =720.60 ft.

The results of the Water Surface Elevations computed by the methods above are plotted on Attachment 2 - "100 YR FLOODPLAIN CROSS-SECTIONS" sheet contained in this report.

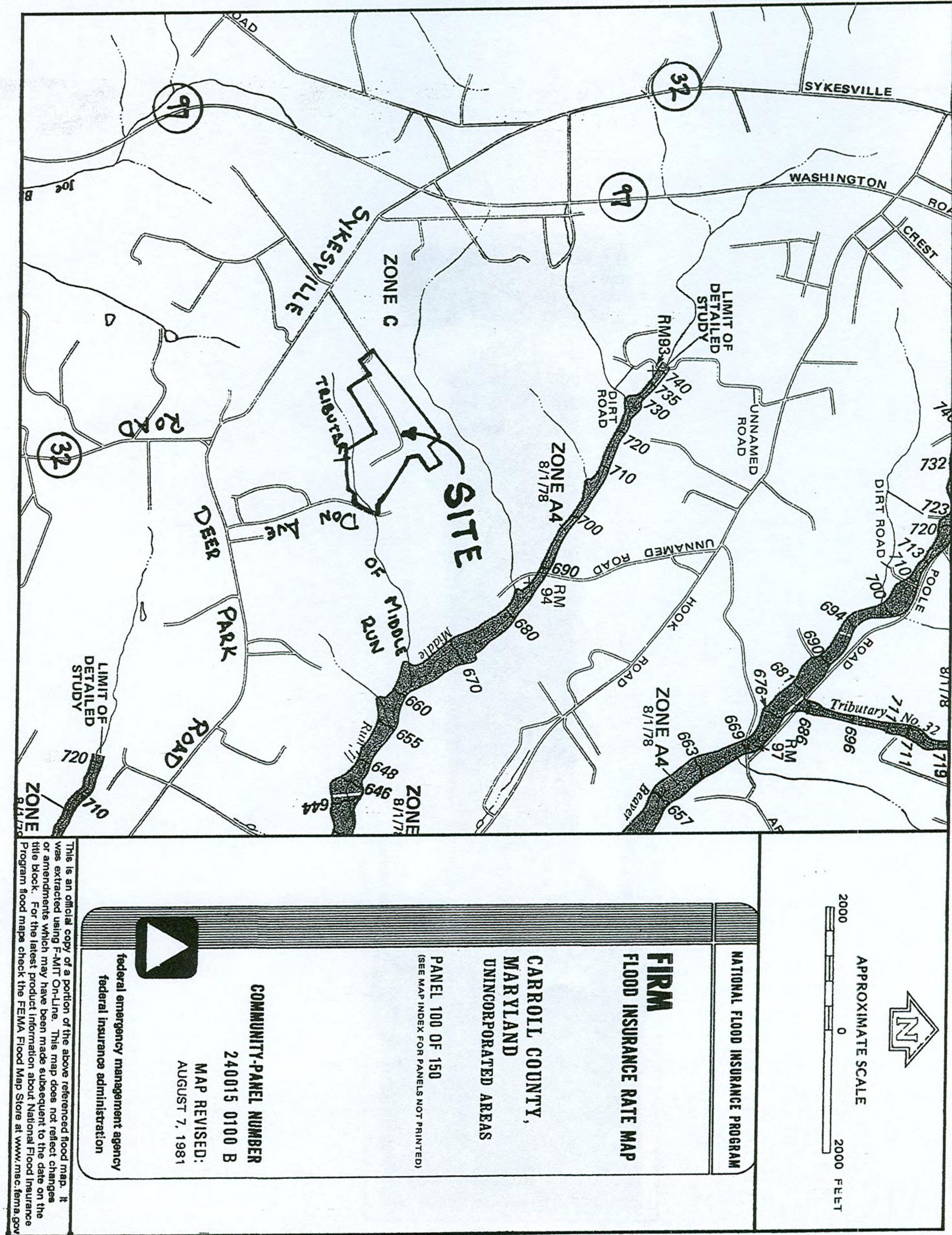


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Pg 4/46



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PROJECT: Wilmot Manor

CC File #P-05-014

TR-55 WORKSHEET 2 Runoff Curve Number and Runoff

DATE: 3/23/06

DESIGN BY: Randall A. Petkus

LOCATION:	Don Avenue @ East Court, 4th E.D. Carroll County, MD.				
CONDITION:	Ultimate Condition				
SUBAREA:	"A"				

* COMPUTED OUTPUT IN BOLD TYPE

<u>1. RUNOFF CURVE NUMBER (CN)</u>

SOIL NAME/

HYDRO GRP	COVER DESCRIPTION	CN			ARE	4	PRODUCT
(appendix)	(Cover Type, Treatment, & Hydrologic Cond.)	Tab 2-2	Fig 2-3	Fig 2-4	(x)ac	<u>()mi</u>	CN x AREA
А	Business Local Zone	89			1.05		93.45
В	Business Local Zone	92			2.01		184.92
Α	R-40000 Zone: 1 ac Lots	51			120.72		6156.72
D	R-40000 Zone: 1 ac Lots	68			19.22		1306.96
С	R-40000 Zone: 1 ac Lots	79			12.23		966.17
							0.00
							0.00
							0.00
							0.00

				0.00
TOTALS			155.2	8708.22
WHEIGHTED CN = product/area =	56.1		USE CN=	56
OFF				
S= 7.86				24
		STORM	STORM	
		#1	#2	
FREQUENCY	Yr.	2	100	
RAINFALL, P(24 hour)	In.	3.1	7.1	
RUNOFF, Q	In.	0.25	2.28	
	WHEIGHTED CN = product/area = OFF S= 7.86 FREQUENCY	WHEIGHTED CN = product/area = 56.1 OFF S= 7.86 FREQUENCY	WHEIGHTED CN = product/area = 56.1 OFF S= 7.86 STORM FREQUENCY Yr. 2 1 RAINFALL, P(24 hour) In. 3.1	WHEIGHTED CN = product/area = 56.1 USE CN= OFF S= 7.86 STORM STORM FREQUENCY Yr. 2 100 RAINFALL, P(24 hour) In. 3.1 7.1

NOTES:

1. For Goundcover delineations & map source information, see DRAINAGE AREA MAP.

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Pg 6/46

216

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PRINTED: 3/23/2006

TR-55 WORKSHEET 3: Time of Concentration(Tc) or Travel Time(Tt)

CHECK BY: R.A. Petkus, P.E. **ORIGIN. DATE:** 3/22/2006

LOCATION: Don Avenue @ East Court 4th E.D. Carroll County, MD. **CONDITION:** Ultimate SUBAREA: "A"

*	COMPUTED	OUTPUT	IN	BOLD	TYPE	

Sheet Flow

Subtotals

	Segmer	nt ID	A-B	
1	Surface Description	F	Residental yard Grasses	
2	Mannings Coeff, n		0.24	
3	Flow Length, L	ft	100.0	
4	2yr-24hr Rainfall, P2	in	3.1	
5	Land Slope, S	'/ft	0.06	
6	Travel Time, Tt	hr	0.156	0.16
Shall	ow Concentrated Flow			

	Segment	ID	B-C	C-D	D-E	
7	Surface Description		unpaved	unpaved	unpaved	
8	Flow Length, L	ft	293.0	722.0	566.0	
9	Watercourse Slope, S	'/ft	0.09	0.07	0.05	
10	Average Velocity, V	ft/s	4.8	4.3	3.6	
11	Travel Time, Tt	hr	0.017	0.047	0.044	0.11
Chan	nel Flow					
	Segment	ID	E-F			
	Assumptions:					
12	X-Sec Flow Area, a	sf				
13	Wetted Perimeter, Pw	ft				
14	Hydraulic Radius, =a/Pw	ft				
15	Channel Slope, S	'/ft	0.02			
16	Mannings Coeff, n		0.04			

20 Watershed or Subarea Tc or Tt..... 0.39 hr

5.0

2287.7

0.127

ft/s

ft

hr

NOTES:

1.

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17 V (Manning formula)

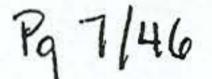
18 Flow Length

19 Travel Time Tt

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PROJECT: Wilmot Manor

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CC File #P-05-014

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CHECK BY:

TR-55 WORKSHEET 4 Graphical Peak Discharge Method

ORIGIN. DATE: 3/20/06 LOCATION: Don Avenue @ East Court 4th E.D. Carroll County, MD. Developed PRES/DEVEL: SUBAREA: "A"

1. DATA:

Drainage Area	
Runoff Curve Number	
Time of Concentration	
Rainfall Distribution Type	
Pond & Swamp Area(s)	

Am=	0.24250	mi^2 155.2 ac
CN=	56	(from worksheet 2)
Tc=	0.36	hr. (from worksheet 3)
=	//	(I, IA, II, III)
=	1.00	% of Am(0 ac or mi ² cov'rd)

		Storm #1	Storm #2	Storm #3	Storm #4
2. FREQUNCY	Yr	2	100		
3. RAINFALL, P(24-hour)	in	3.1	7.1		
4. INITIAL ABSTRACTION, Ia (la =200/CN-2)	in	1.571	1.571		
5. COMPUTE la/P		0.51	0.22		
6. UNIT PEAK DISCHARGE, qu (Use Tc and Ia/P with Exhibit 4-II)	csm /in	270.0	580.0		
7. RUNOFF, Q (from worksheet 2)	in	0.25	2.28		
FACTOR (use % pond/swamp area w/ table 4-2)	Fp	1.00	1.00		
9. PEAK DISCHARGE, qp where qp = qu x Am x Q x Fp)	cfs	16.37	320.68		

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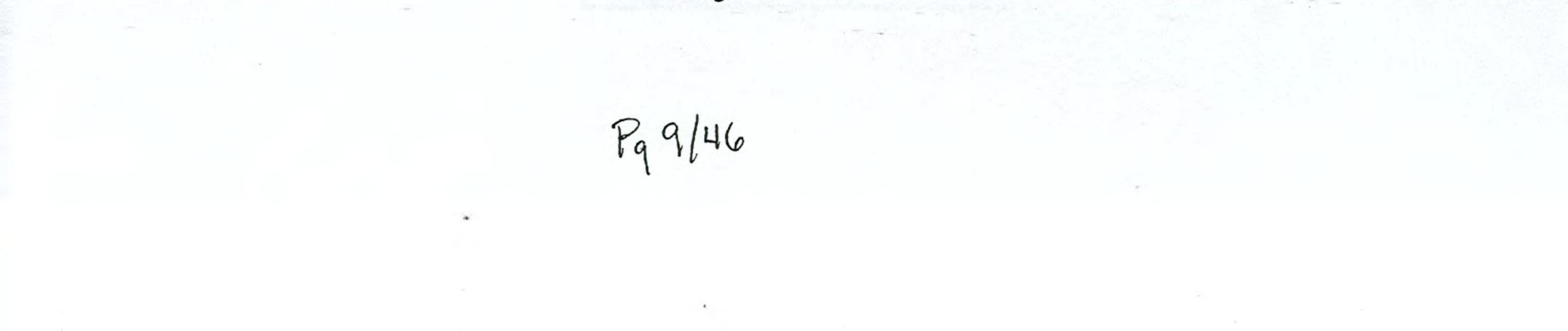
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	³ NO. ³ (ft) ³ 1 ³ 713.50	(ft) 710.50	(ft)) 105.0	³ MAT 4 ³ 1 R	CP	(ft) 3.00	(ft) 3.00	n .012	CONVENTIO	DNAL 3
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	3 6 3			3	*******		******	******	*******	3 (XXXI)
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	713.50 716.14	0.0 32.1	0.0 32.1	0.0	0.0	0.0	0.0	0.0	0.00	1
	718.13 719.84	64.1 96.2	64.1 82.3	0.0	0.0	0.0	0.0	0.0	0.00 13.20	8
	719.87	100.0	82.7	0.0	0.0	0.0	0.0	0.0	16.60	6
	720.18 720.28	160.4 192.4	85.5 86.4	0.0	0.0	0.0	0.0	0.0	104.39	4
	720.37 720.45	224.5 256.6	87.2 87.9	0.0	0.0	0.0	0.0	0.0	135.80 167.35	4
	720.53	288.6	88.6	0.0	0.0	0.0	0.0	0.0	198.89	4
	720.60	320.7 78.8	89.2	0.0	0.0	0.0	0.0	0.0	230.46 OVERTOPPI	4 NG
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	HEAD		HEAD		TOTAL		FLOW		% FLOW	
	ELEV		ERROR (f		FLOW (C		ERROR ((cfs)	ERROR	
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	718.13		0.000		64.14		0.00		0.00	
	719.84 719.87		-0.004		96.21 100.00		0.68		0.71 0.74	
	720.18	3	-0.004		160.35		0.63		0.39	
	720.28		-0.009		192.42 224.49		1.67		0.87 0.68	
	720.45	5	-0.007		256.56		1.30		0.51	
	720.53		-0.006 -0.005		288.63 320.70		$1.16 \\ 1.03$		0.40 0.32	
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	FLOW ELEV.	DEPTH	DEPTH	TYPE	DEPTH	DEPTH	DEPTH	DEPTH	VEL.	VEL.
	(cfs) (ft)	(ft)	(ft)	<f4></f4>	(ft)	(ft)	(ft)	(ft)	(fps)	(fps)
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	32.07 716.14	2.64		1-S1f	1.04	1.83	2.70	3.00	9.57	0.00
	64.14 718.13	4.63		1-S1f	1.80	2.84	1.80	3.00	18.58	0.00
	82.33 719.83	6.33		4-FFt 4-FFt	1.80	2.85	1.81	3.00	18.59	0.00
	82.66 719.87 85.47 720.18	6.37	4.18		1.85	2.89	1.85	3.00	18.70	0.00
	85.47 720.18 86.36 720.27	6.77	4.27		1.86	2.91	1.86	3.00	18.77	0.00
	87.18 720.37	6.87	4.35		1.87	2.92	1.87	3.00	18.80	0.00
	87.91 720.45	6.95	4.42		1.88	2.93	1.88	3.00	18.84	0.00
	88.59 720.53	7.03	4.49		1.89	2.94	1.89	3.00	18.87	0.00
	89 21 720.60	7.10	4.56	4-FFt	1.90	2.95	1.90	3.00	18.89	0.00
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	SLOPE (V/H)					0.028				

SLOPE (V/H) CULVERT LENGTH ALONG SLOPE 105

105.04 ft

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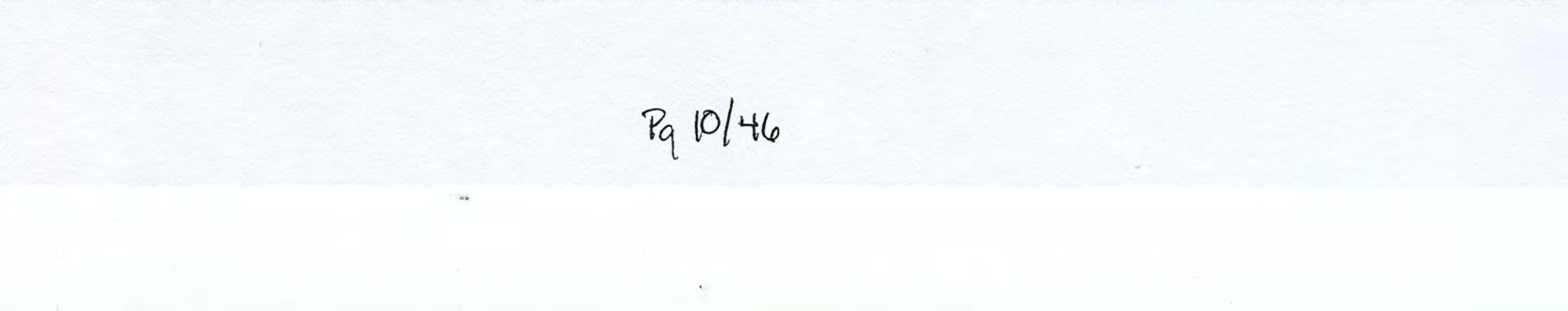
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CONSTANT WATER SURFACE ELEVATION 713.50

ROADWAY SURFACE

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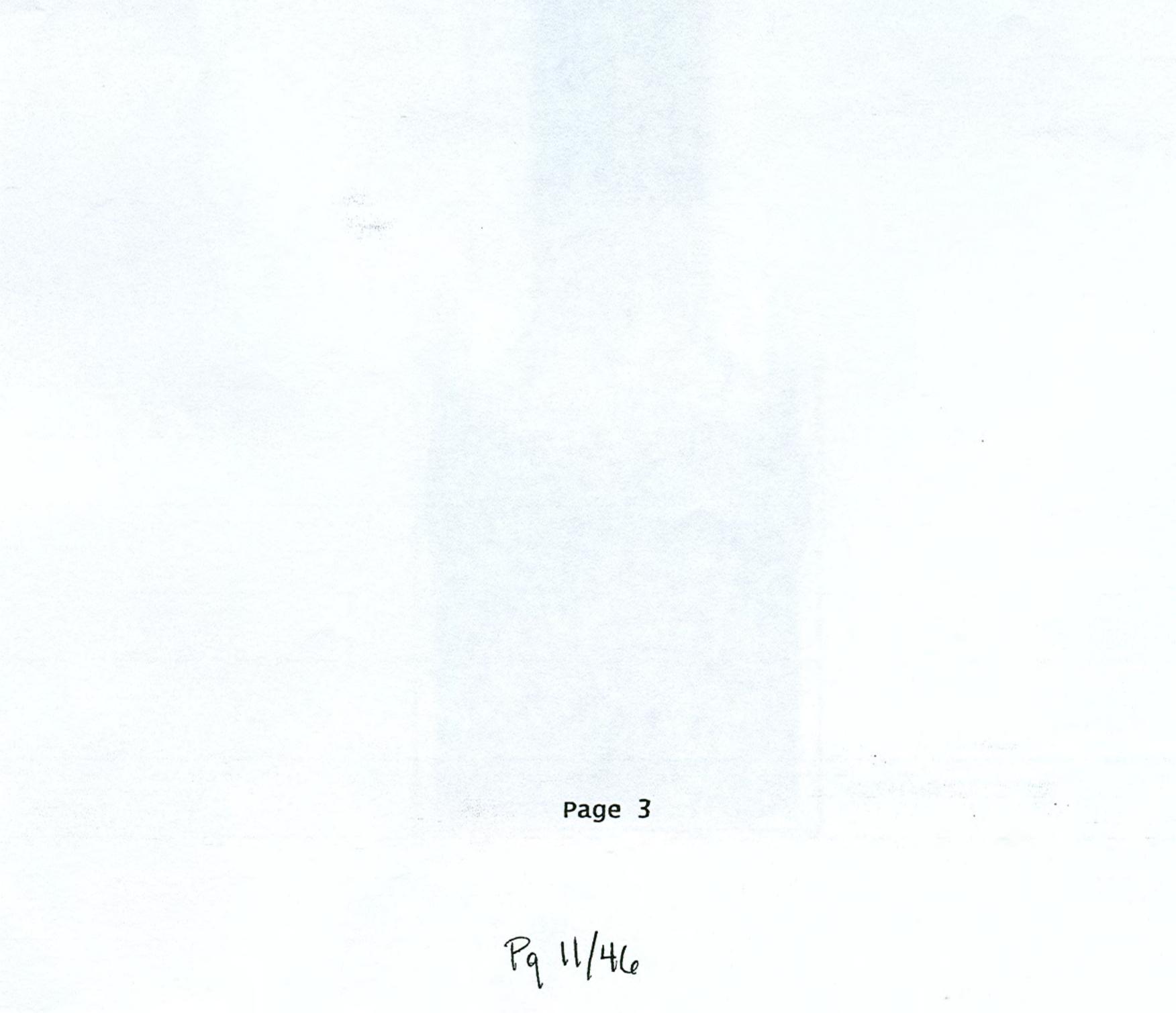
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****	USER DEFINED RO	DADWAY PRO	OFILE		
	CROSS-SECTION	X	Y		
	COORD. NO.	ft	ft		
	1	0.00	721.23		
	2	43.08	720.15		
	3	93.79	719.47		
	4	142.70	720.07		
	5	191.80	722.74		
	6	236.75	726.04		
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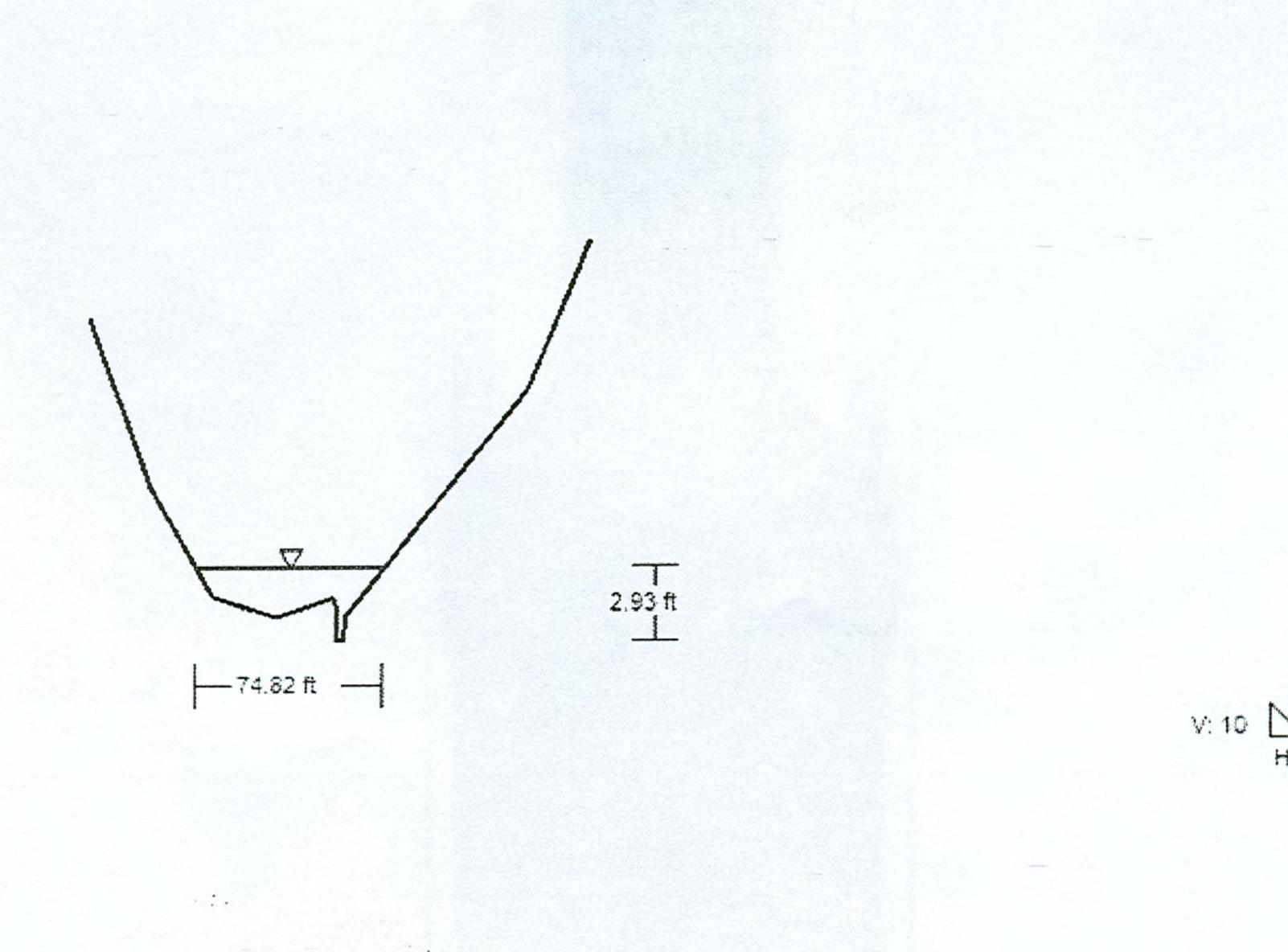
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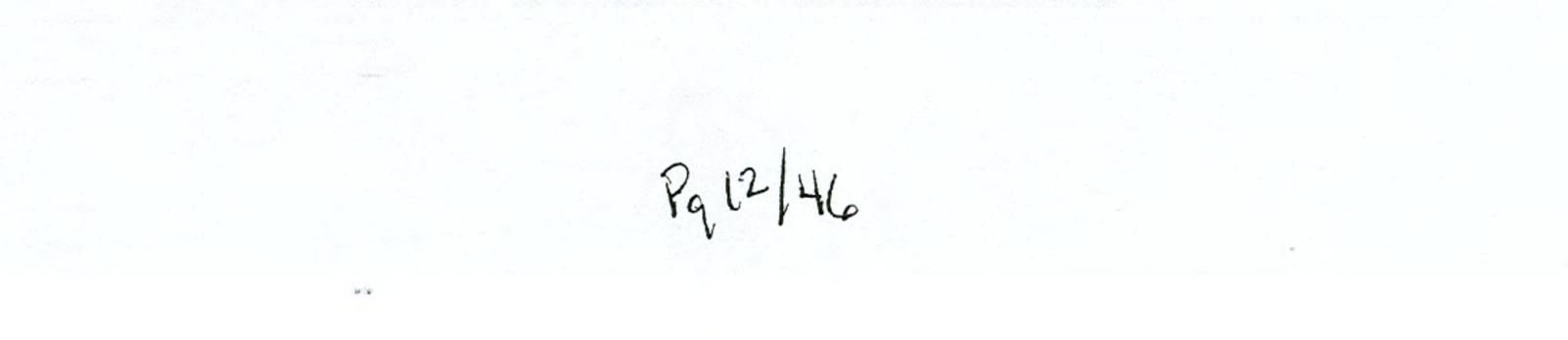


XS No.10 @ RS 105+50 Cross Section for Tributary No.2 - XS No.10 @ RS 105+50

1922

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.087	
Channel Slope:	0.02081	ft/ft
Normal Depth:	2.93	ft
Elevation Range:	735.62 to 751.60 ft	
Discharge:	320.68	ft³/s





Worksheet for Tributary No.2 - XS No.10 @ RS 105+50

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Input Data	0.00004	a /a
Channel Slope:	0.02081	ft/ft
Discharge:	320.68	ft³/s
Options		
	Improved attors	
Current Roughness Weighted Metho	ImprovedLotters	
Open Channel Weighted Roughnes:	ImprovedLotters	
Closed Channel Weighted Roughne	Hortons	
Results		
Roughness Coefficient:	0.087	
Water Surface Elevation:	738.55	ft
Elevation Range:	735.62 to 751.60 ft	
Flow Area:	105.43	ft²
Wetted Perimeter:	76.67	ft
Top Width:	74.82	ft

2.93 Normal Depth:

Homai Dopin		
Critical Depth:	2.28	ft
Critical Slope:	0.11852	ft/ft
Velocity:	3.04	ft/s
Velocity Head:	0.14	ft
Specific Energy:	3.08 -	ft
Froude Number:	0.45	
Flow Type:	Subcritical	

Segment Roughness

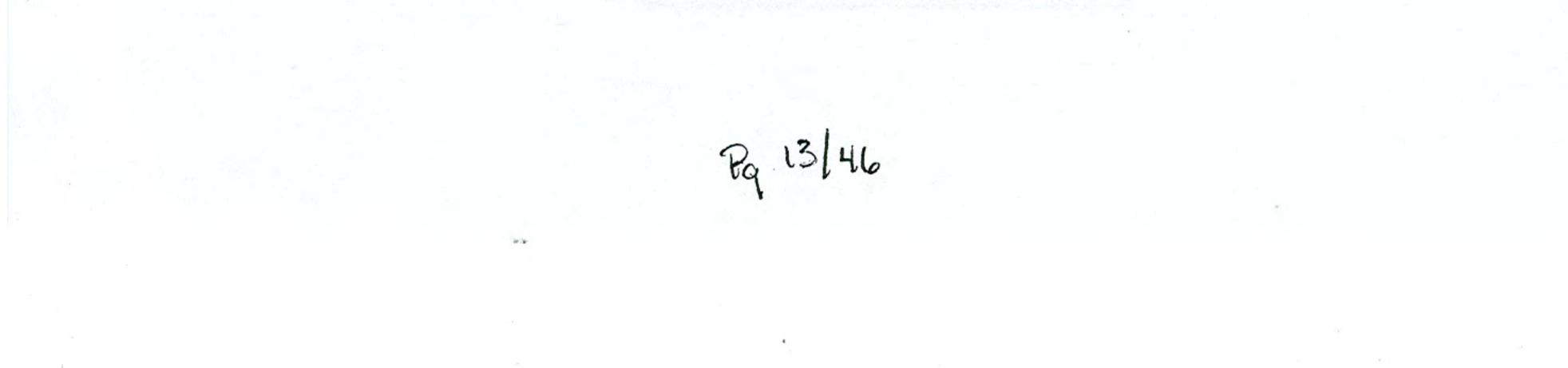
Start Station	End Station	Roughness Coefficient
(0+00.00, 748.50)	(0+98.40, 737.37)	0.100
(0+98.40, 737.37)	(1+02.51, 736.62)	0.050
(1+02.51, 736.62)	(2+00.00, 751.60)	0.100

Section Geometry

ft S

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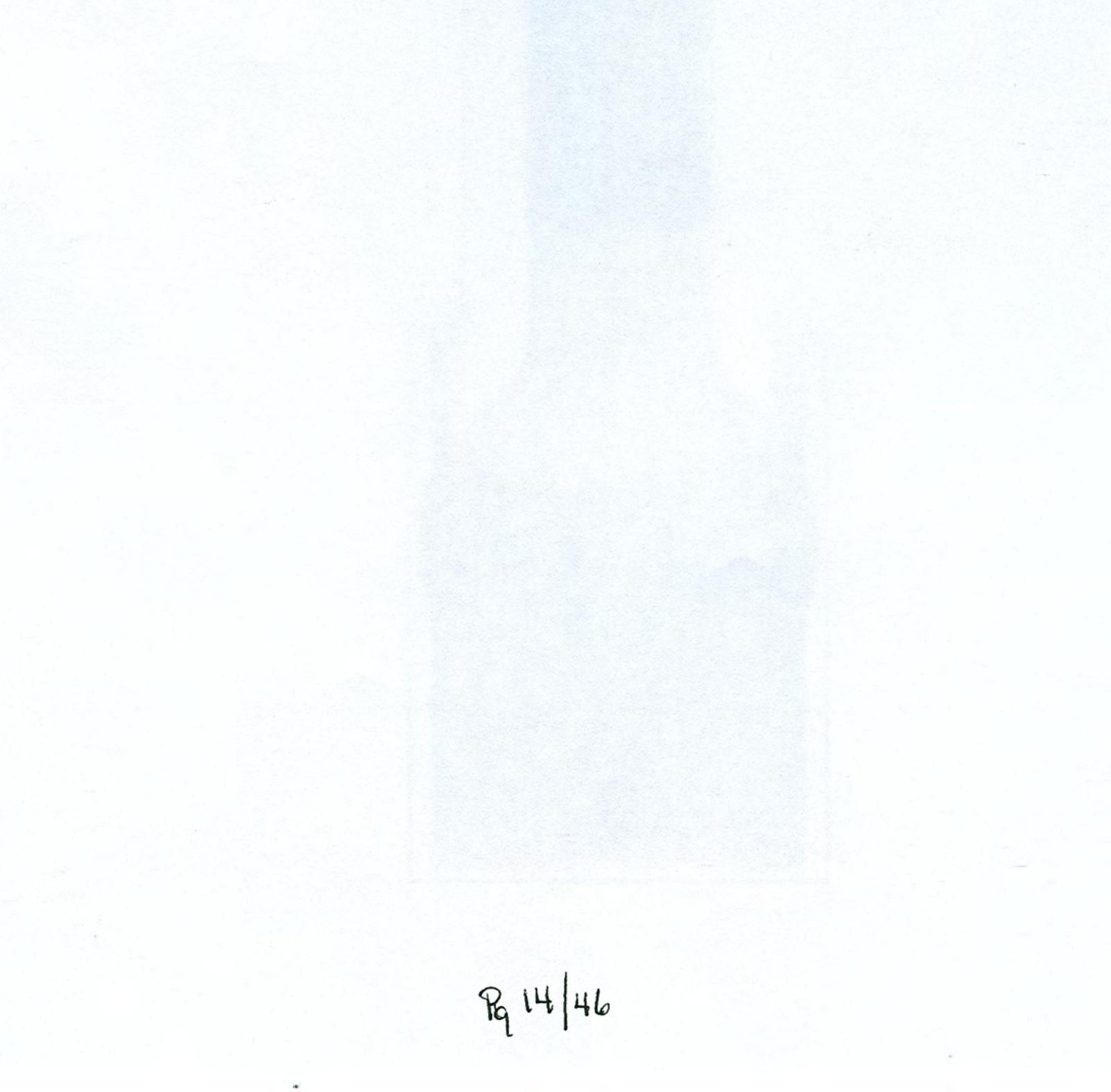
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Worksheet for Tributary No.2 - XS No.10 @ RS 105+50

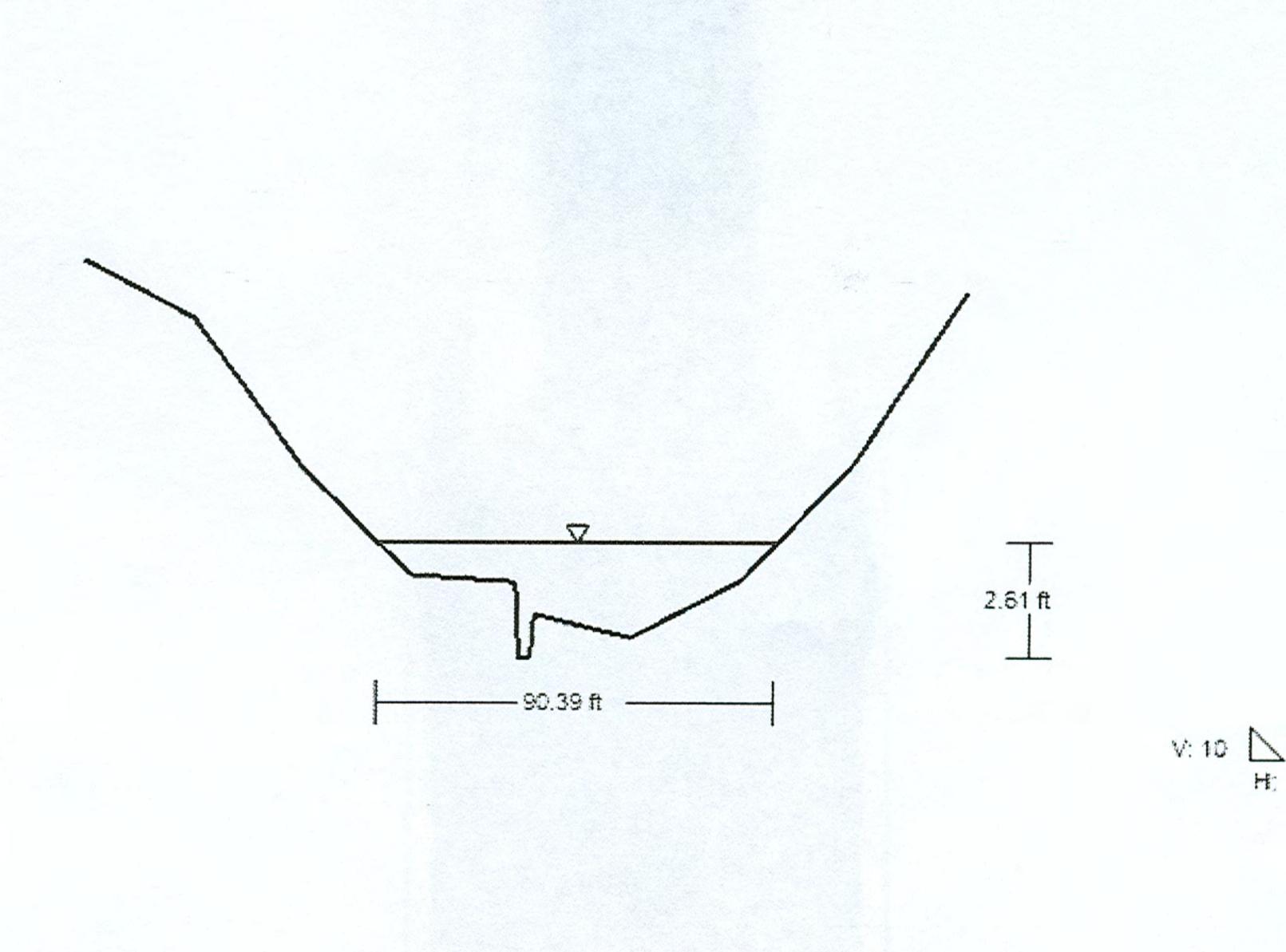
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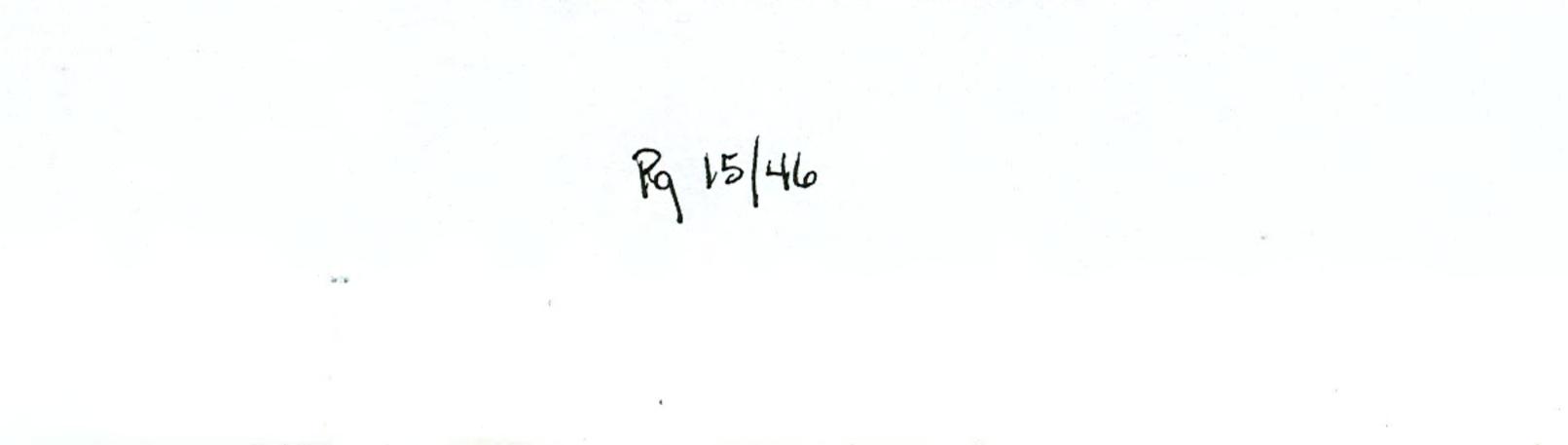
Station	Elevation	
0+00.00	748.50	
0+25.00	741.70	
0+50.00	737.40	
0+75.00	736.60	
0+98.40	737.37	
0+99.20	735.62	
1+01.70	735.62	
1+02.51	736.62	
1+75.00	745.60	
2+00.00	751.60	



XS No.9 @ RS 103+00 Cross Section for Tributary No.2 - XS No.9 @ RS 103+00

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.086	
Channel Slope:	0.02081	ft/ft
Normal Depth:	2.61	ft
Elevation Range:	730.41 to 739.40 ft	
Discharge:	320.68	ft³/s





Worksheet for Tributary No.2 - XS No.9 @ RS 103+00

Project Description			
Flow Element:	Irregular Section		
Friction Method:	Manning Formula		
Solve For:	Normal Depth		
Input Data			
Channel Slope:	0.02081	ft/ft	
Discharge:	320.68	ft³/s	
Options			
Current Roughness Weighted Metho	ImprovedLotters		
Open Channel Weighted Roughnes:	ImprovedLotters		
Closed Channel Weighted Roughne	Hortons		
Results			
Roughness Coefficient:	0.086		
Water Surface Elevation:	733.02	ft	
Elevation Range:	730.41 to 739.40 ft		
Flow Area:	112.41	ft²	
Wetted Perimeter:	92.09	ft	
Top Width:	90.39	ft	

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Normal Depth:	2.61
Critical Depth:	2.03
Critical Slope:	0.11872
Velocity:	2.85
Velocity Head:	0.13
Specific Energy:	2.74
Froude Number:	0.45
Flow Type:	Subcritical

Segment Roughness

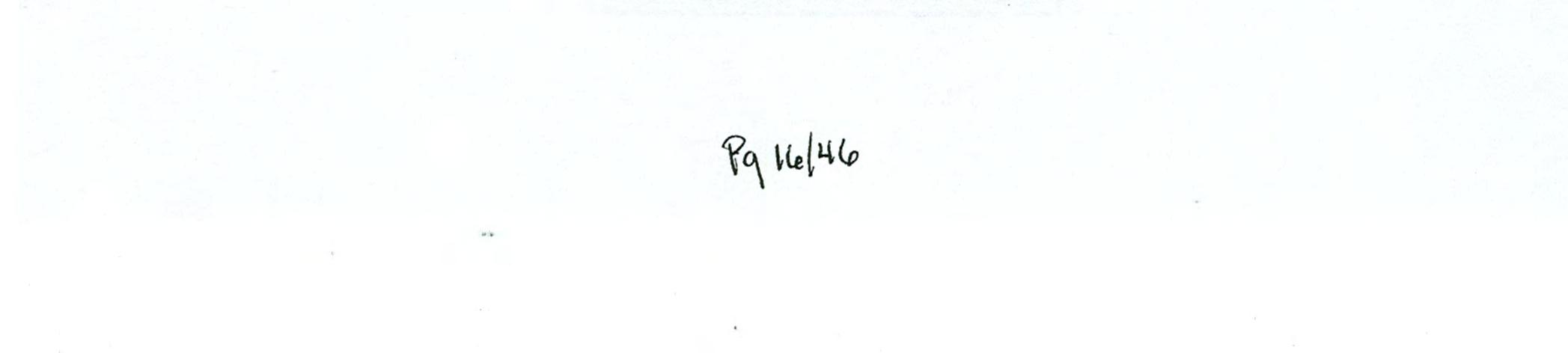
States States and

Start Station	End Station	Roughness Coefficient
(0+00.00, 739.40)	(0+98.77, 732.15)	0.100
(0+98.77, 732.15)	(1+02.96, 731.42)	0.050
(1+02.96, 731.42)	(2+00.00, 738.70)	0.100

Section Geometry

ft ft/ft ft/s ft ft

ft



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Worksheet for Tributary No.2 - XS No.9 @ RS 103+00

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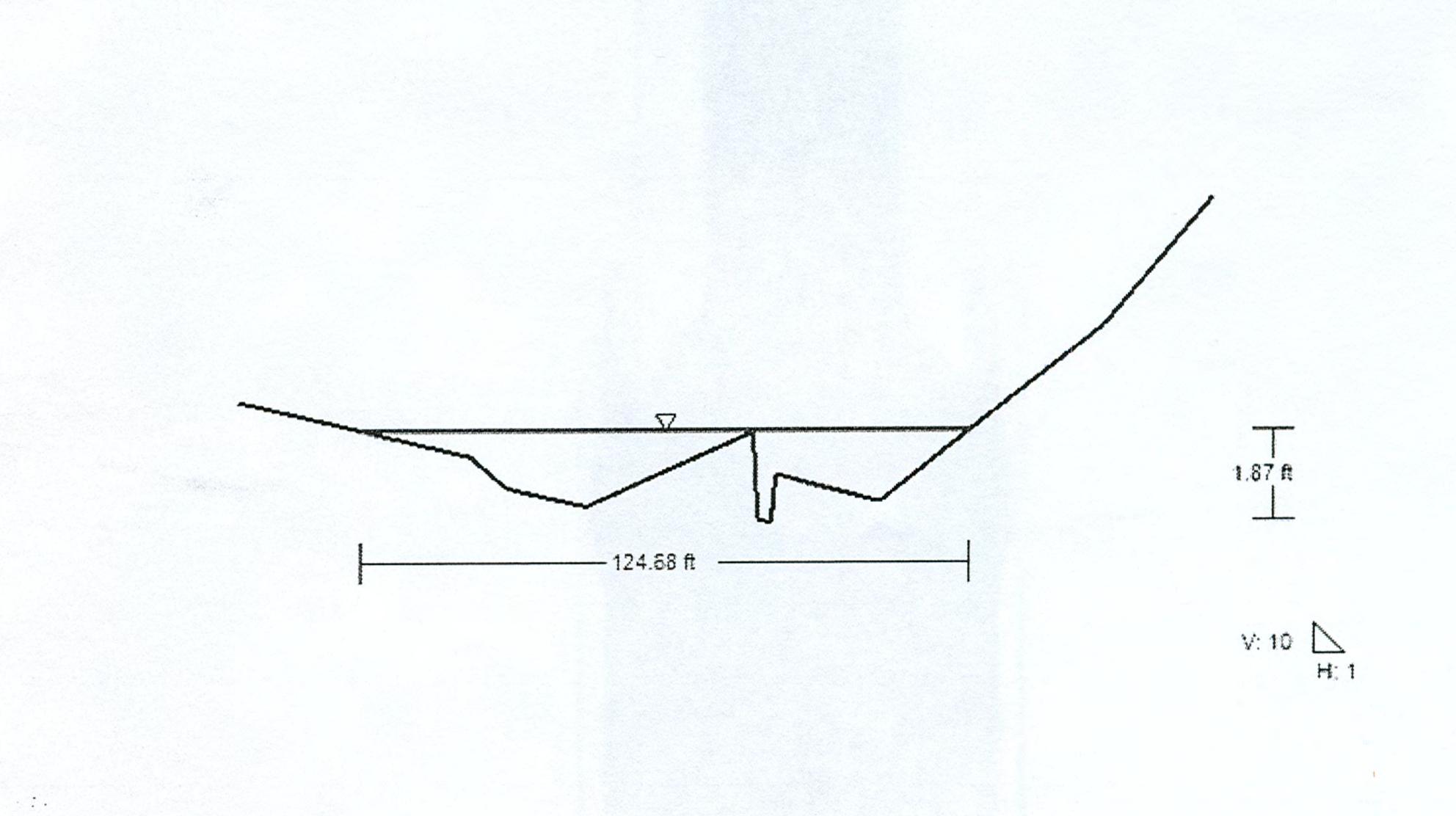
Station	Elevation	•
0+00.00	739.40	
0+25.00	738.10	
0+50.00	734.70	
0+75.00	732.30	
0+98.77	732.15	
0+99.58	730.41	
1+02.14	730.42	
1+02.96	731.42	
1+25.00	730.90	
1+50.00	732.20	
1+75.00	734.80	
2+00.00	738.70	

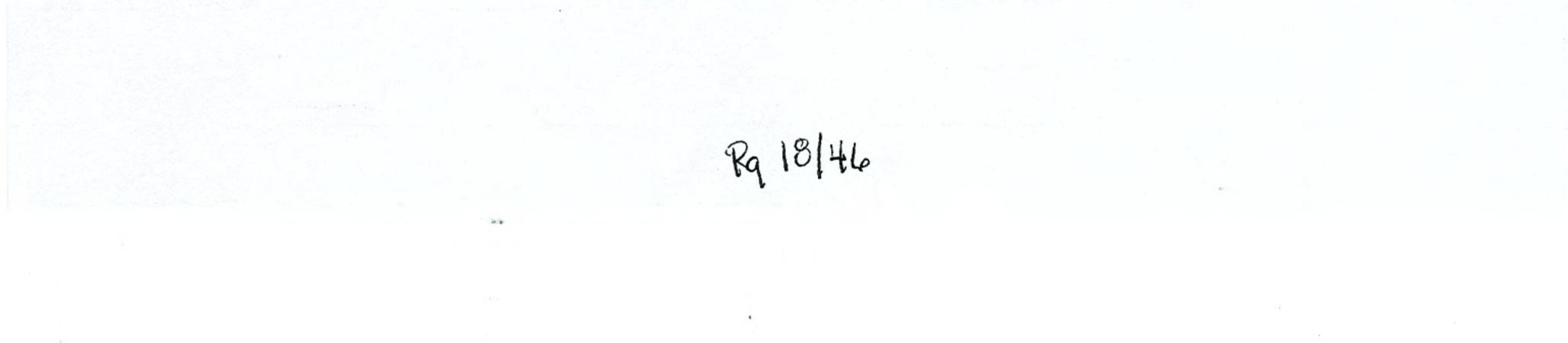


XS No.8 @ RS 102+00 Cross Section for Tributary No.2 - XS No.8 @ RS 102+00

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.091	
Channel Slope:	0.04337	ft/ft
Normal Depth:	1.87	ft
Elevation Range:	726.29 to 732.94 ft	
Discharge:	320.68	ft³/s

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Worksheet for Tributary No.2 - XS No.8 @ RS 102+00

	Project Description	
	Flow Element:	Irregular Section
	Friction Method:	Manning Formula
	Solve For:	Normal Depth
pittings"	Input Data	
	Channel Slope:	0.04337
	Discharge:	320.68
	Options	
	Current Roughness Weighted Metho	ImprovedLotters
	Open Channel Weighted Roughnes:	ImprovedLotters
	Closed Channel Weighted Roughne	Hortons
	Results	
	Roughness Coefficient:	0.091
	Water Surface Elevation:	728.16
	Elevation Range:	726.29 to 732.94 ft
	Flow Area:	106.16
	Wetted Perimeter:	126.33
	Top Width:	124.68

Top Width: 124.68 Normal Depth: 1.87

Normal Depth.	1.07
Critical Depth:	1.53
Critical Slope:	0.13998
Velocity:	3.02
Velocity Head:	0.14
Specific Energy:	2.01
Froude Number:	0.58
Flow Type:	Subcritical

Segment Roughness

End Station	Roughness Coefficient
(1+06.03, 728.13)	0.100
(1+10.63, 727.27)	0.050
(2+00.00, 732.94)	0.100
	(1+06.03, 728.13) (1+10.63, 727.27) (2+00.00,

Section Geometry

ft ft/ft ft/s ft ft

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ft/ft

ft³/s

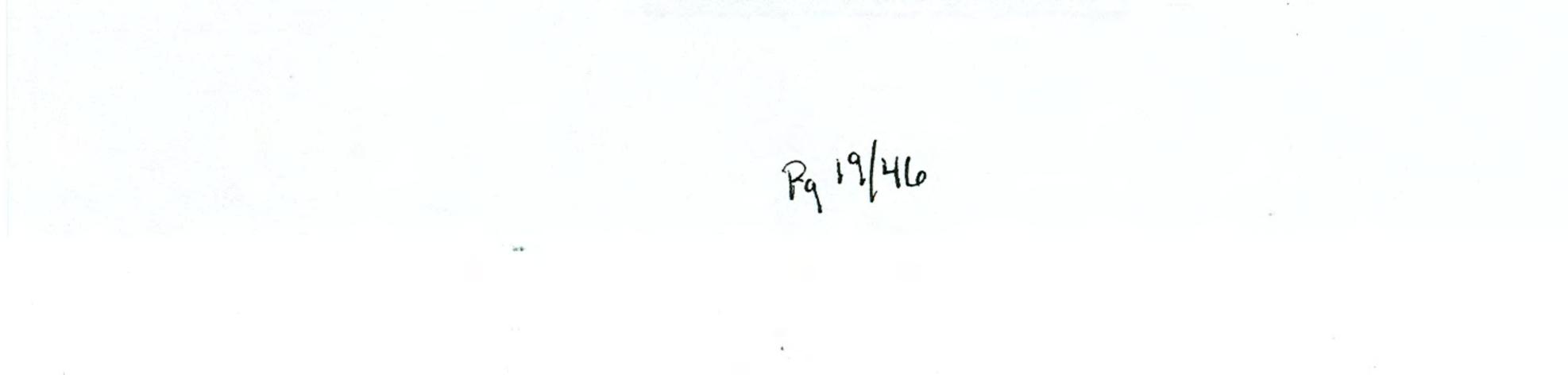
ft

ft²

ft

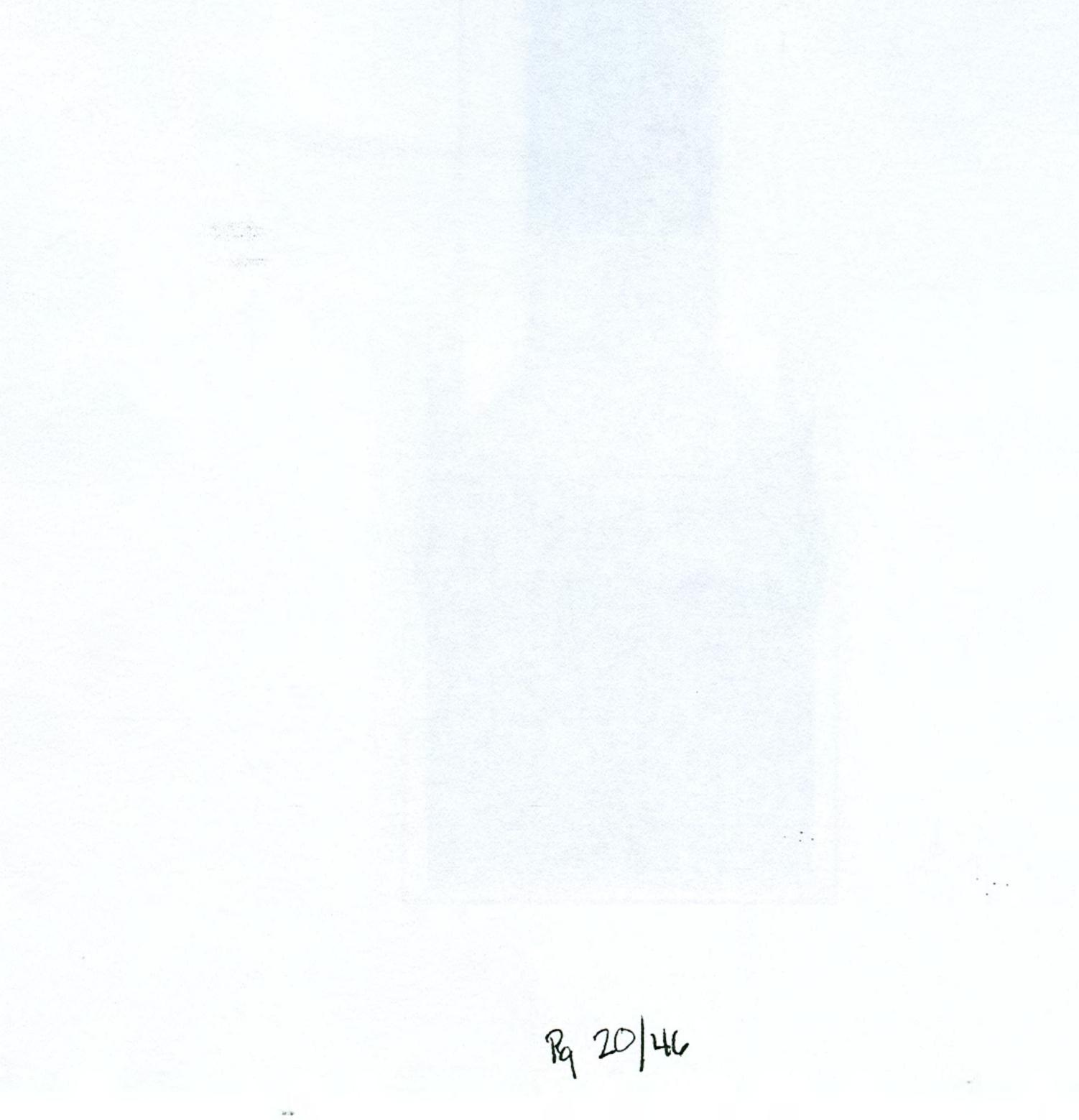
ft

ft



Worksheet for Tributary No.2 - XS No.8 @ RS 102+00

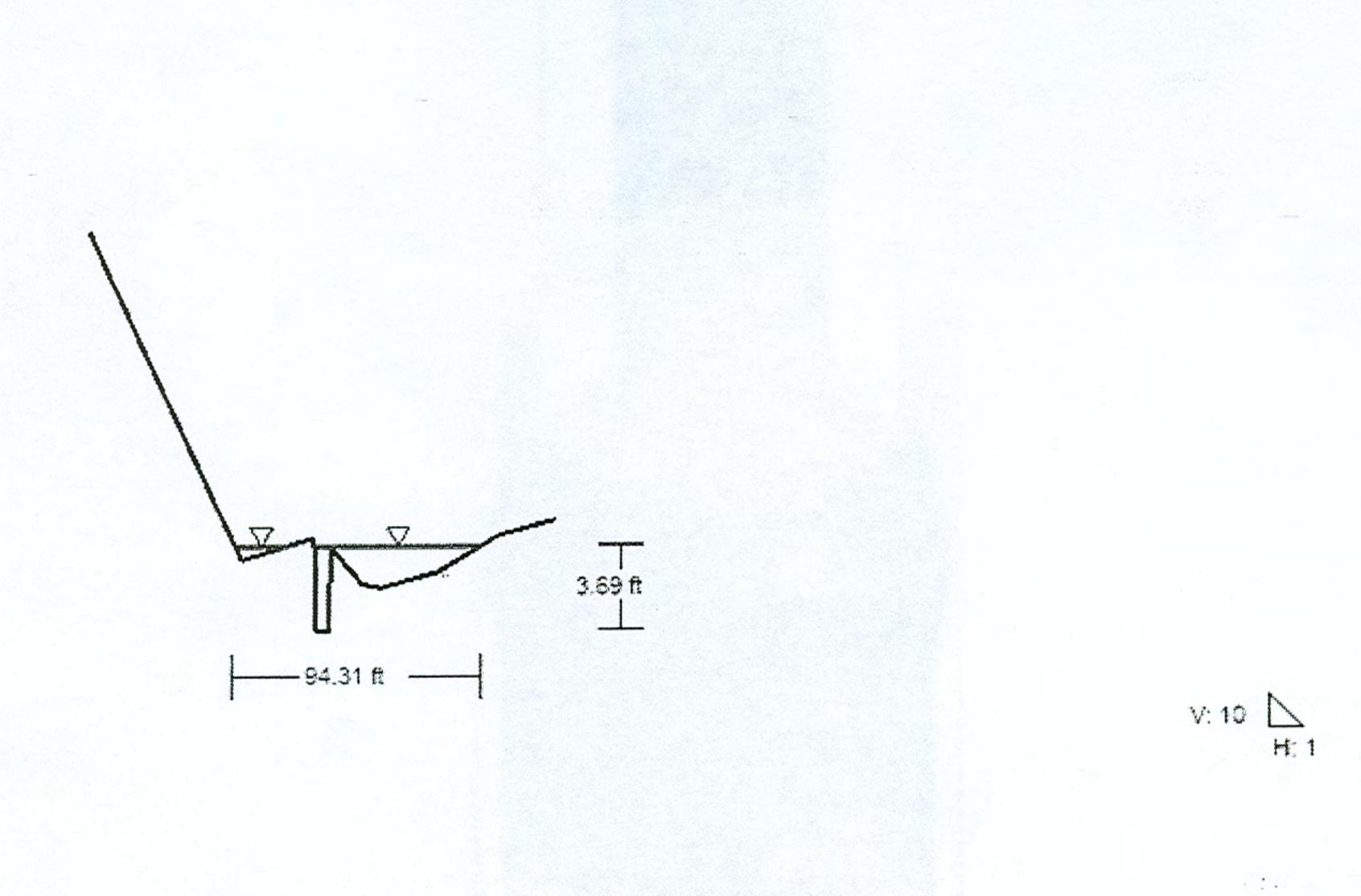
Station	Elevation
0+00.00	728.77
0+47.89	727.64
0+55.17	727.01
0+71.71	726.63
1+06.03	728.13
1+06.93	726.36
1+09.73	726.29
1+10.63	727.27
1+32.14	726.73
1+77.76	730.29
2+00.00	732.94

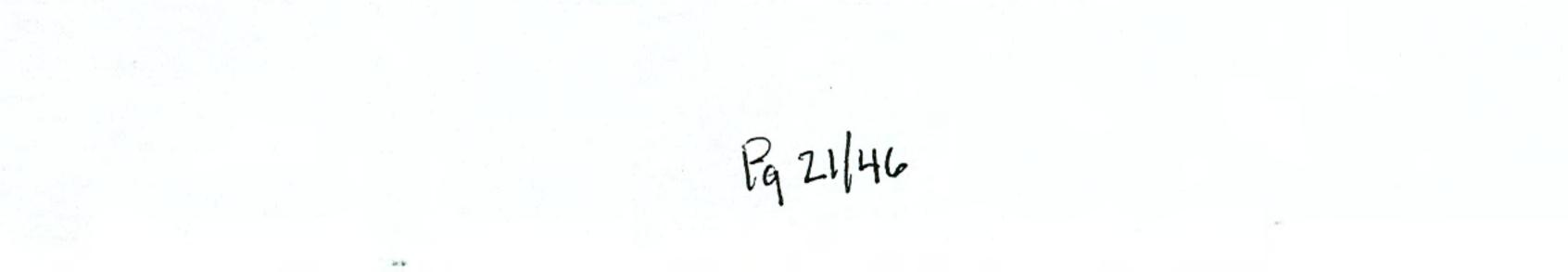


XS No.7 @ RS 26+50 Cross Section for Tributary No.1 - XS No.7 @ RS 26+50

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.069	
Channel Slope:	0.02173	ft/ft
Normal Depth:	3.69	ft
Elevation Range:	746.81 to 763.90 ft	
Discharge:	320.68	ft³/s

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Worksheet for Tributary No.1 - XS No.7 @ RS 26+50

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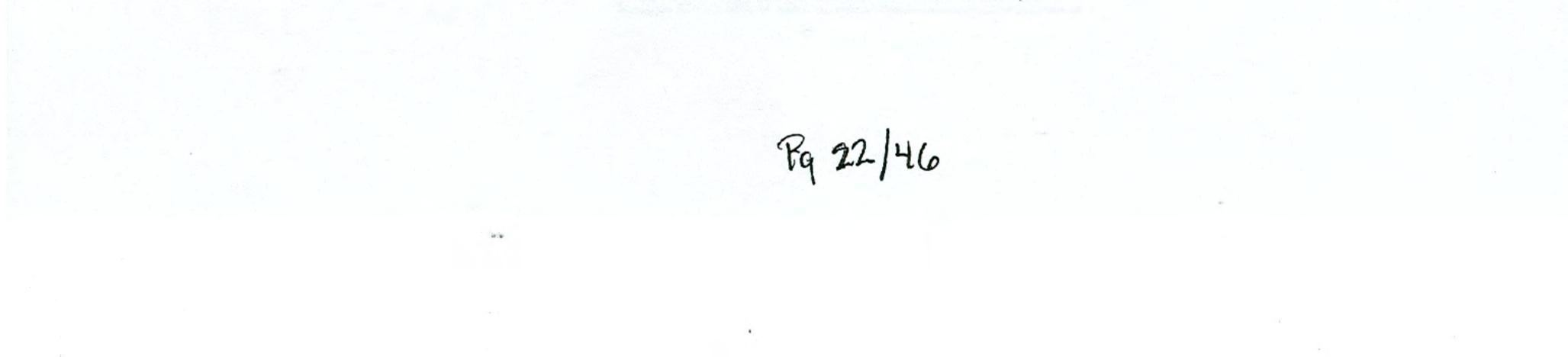
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	Project Description		
	Flow Element:	Irregular Section	
	Friction Method:	Manning Formula	
	Solve For:	Normal Depth	
	Input Data	0.00170	6.1G
	Channel Slope:	0.02173	ft/ft
	Discharge:	320.68	ft³/s
	Options		
	Current Roughness Weighted Metho	ImprovedLotters	
	Open Channel Weighted Roughnes:	ImprovedLotters	
	Closed Channel Weighted Roughne	Hortons	
	Results Revelations	0.069	
	Roughness Coefficient:		A
	Water Surface Elevation:	750.50	ft
	Elevation Range:	746.81 to 763.90 ft	
	Flow Area:	100.85	ft²
	Wetted Perimeter:	100.10	ft
-	Top Width:	94.31	ft
1	Normal Depth:	3.69	ft
	Critical Depth:	3.13	ft
(Critical Slope:	0.07834	ft/ft
'	Velocity:	3.18	ft/s
١	Velocity Head:	0.16	ft
:	Specific Energy:	3.85	ft
F	Froude Number:	0.54	
F	Flow Type:	Subcritical	

Segment Roughness

End Station	Roughness Coefficient
(0+96.67, 750.83)	0.100
(1+03.90, 750.41)	0.050
(2+00.00, 751.60)	0.100
	(0+96.67, 750.83) (1+03.90, 750.41) (2+00.00,

Section Geometry

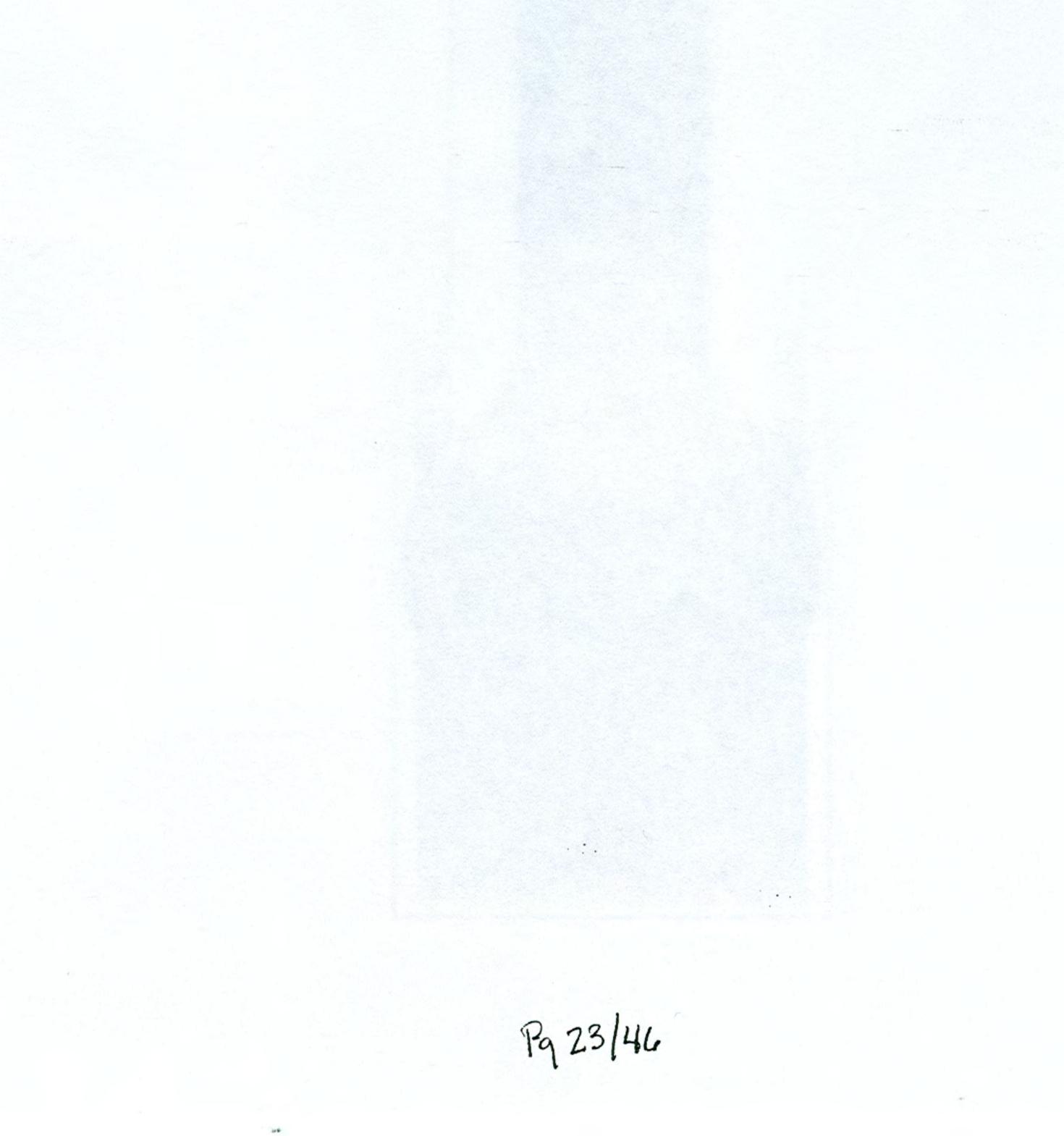


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Worksheet for Tributary No.1 - XS No.7 @ RS 26+50

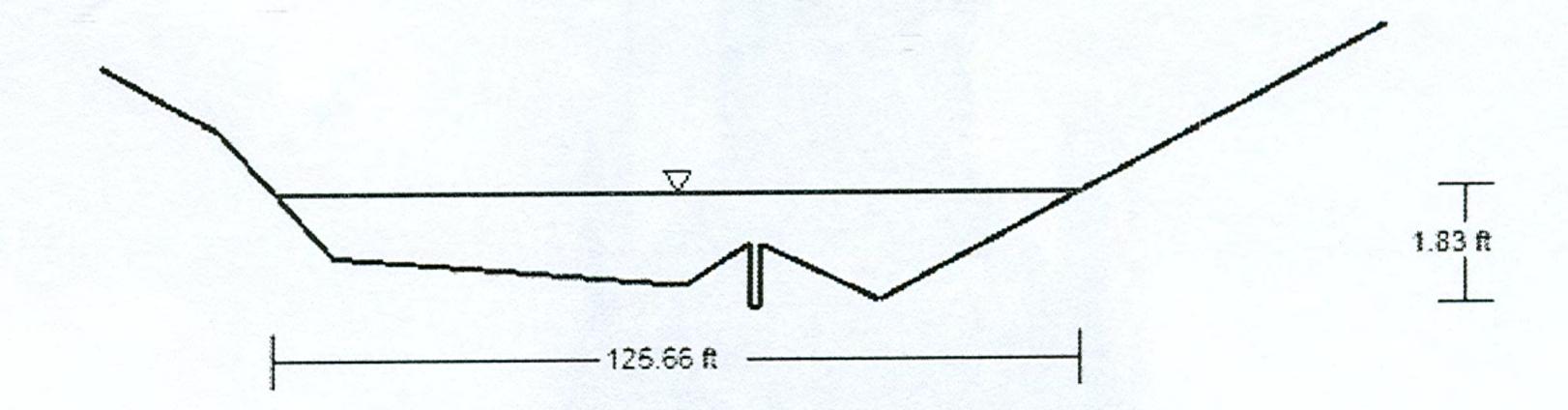
Station	Elevation	
0+00.00	763.90	
0+65.73	749.91	
0+96.67	750.83	
0+97.68	746.83	
1+02.89	746.81	
1+03.90	750.41	
1+17.39	748.85	
1+25.00	748.70	
1+50.00	749.40	
1+75.00	750.90	
2+00.00	751.60	

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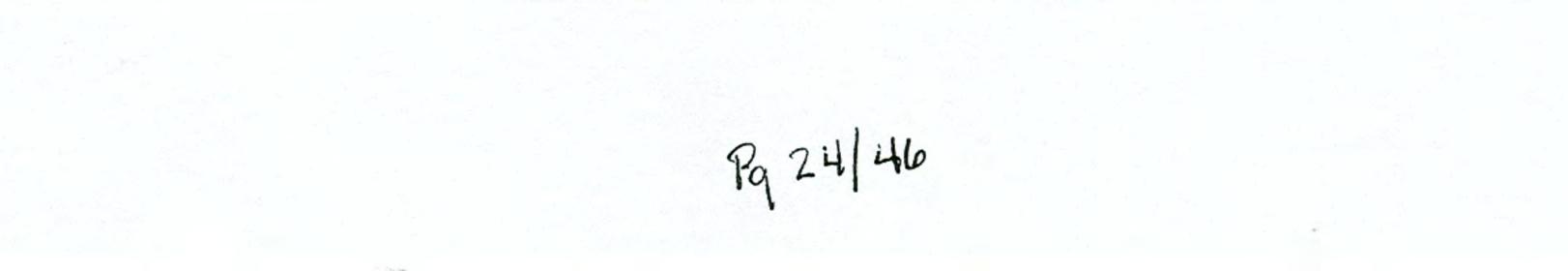


XS No.6 @ RS 22+75 Cross Section for Tributary No.1 - XS No.6 @ RS 22+75

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.098	
Channel Slope:	0.02173	ft/ft
Normal Depth:	1.83	ft
Elevation Range:	738.71 to 743.08 ft	
Discharge:	320.68	ft³/s



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Worksheet for Tributary No.1 - XS No.6 @ RS 22+75

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Input Data		
Channel Slope:	0.02173	ft/ft
Discharge:	320.68	ft³/s
Options		
Current Roughness Weighted Metho	ImprovedLotters	
Open Channel Weighted Roughnes:	ImprovedLotters	
Closed Channel Weighted Roughne	Hortons	
Results		
Roughness Coefficient:	0.098	
Water Surface Elevation:	740.54	ft
Elevation Range:	738.71 to 743.08 ft	
Flow Area:	136.72	ft²
Wetted Perimeter:	127.56	ft
Top Width:	125.66	ft
Normal Depth:	1.83	ft
Critical Depth:	1.27	ft
Critical Slope:	0.16526	ft/ft
Velocity:	2.35	ft/s
Velocity Head:	0.09	ft
Specific Energy:	1.91	ft
Froude Number:	0.40	
Flow Type:	Subcritical	

Segment Roughness

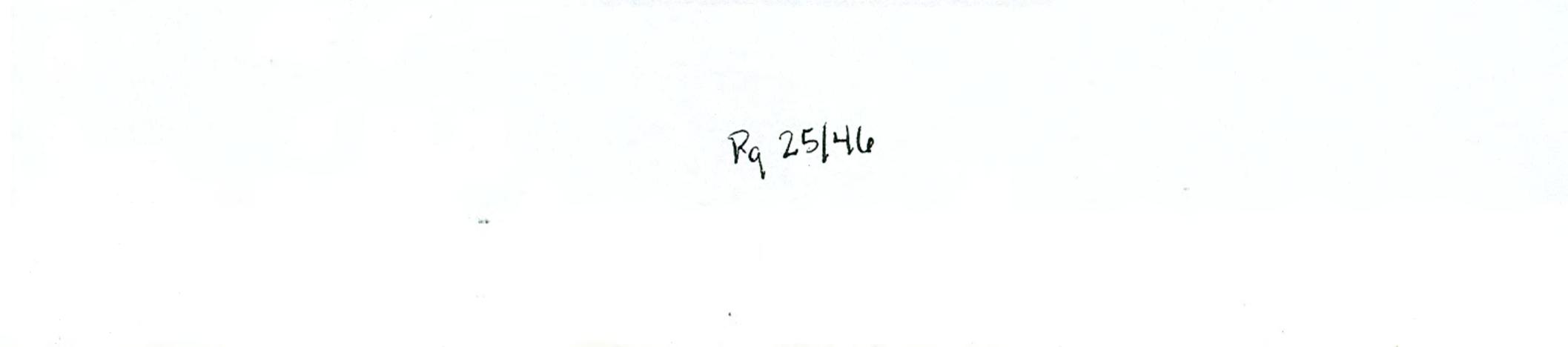
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Start Station	End Station	Roughness Coefficient
(0+00.00, 742.52)	(1+01.51, 739.74)	0.100
(1+01.51, 739.74)	(1+03.32, 739.71)	0.050
(1+03.32, 739.71)	(2+00.00, 743.08)	0.100

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Section Geometry

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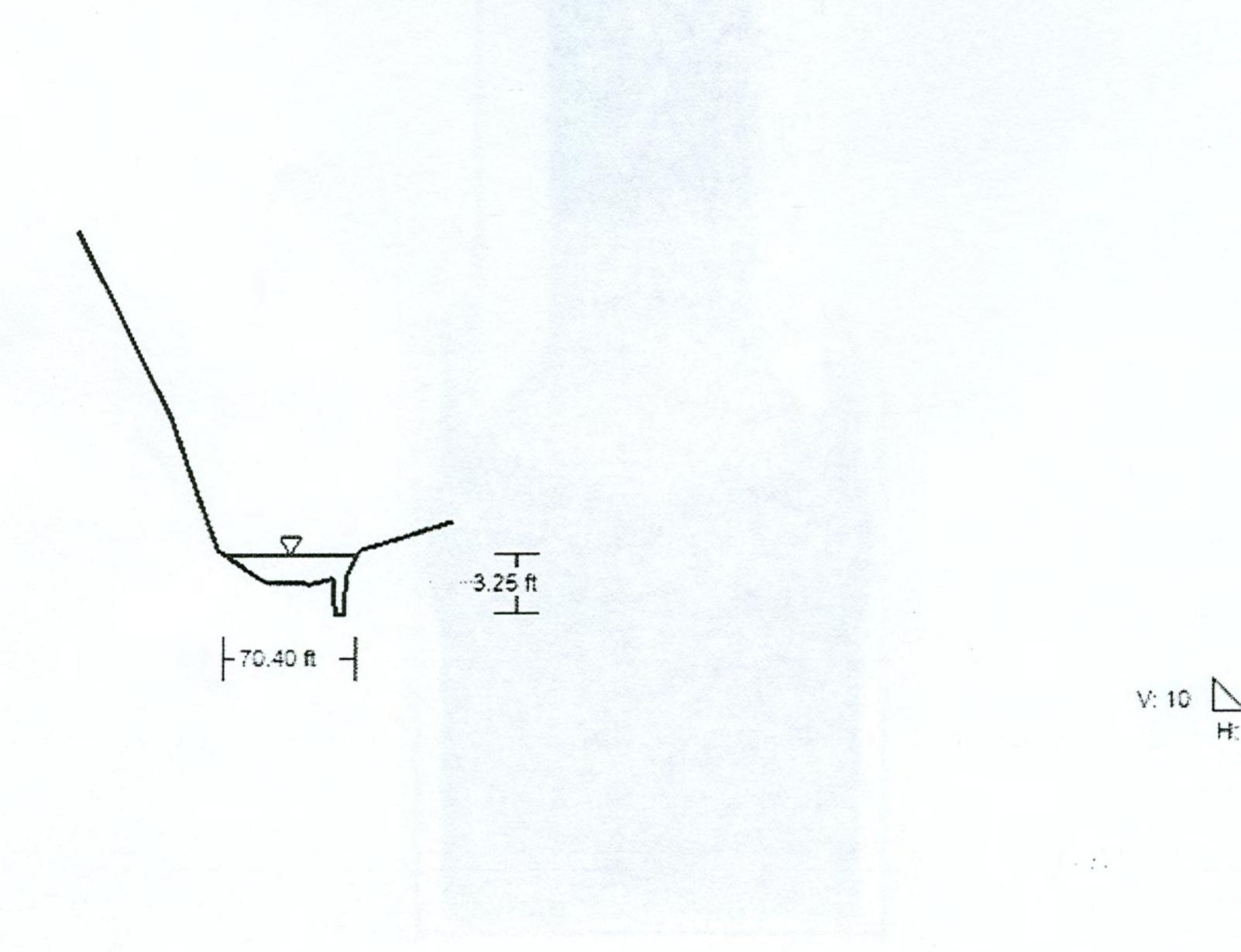
Worksheet for Tributary No.1 - XS No.6 @ RS 22+75

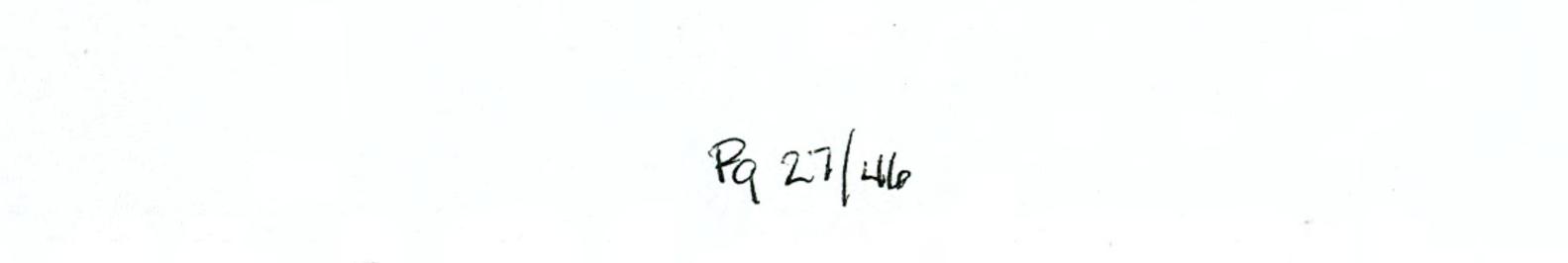
Station	Elevation
0+00.00	742.52
0+18.55	741.50
0+36.56	739.52
0+91.31	739.08
1+01.51	739.74
1+01.64	738.74
1+03.19	738.71
1+03.32	739.71
1+21.61	738.84
2+00.00	743.08



XS No.5 @ RS 19+50 Cross Section for Tributary No.1 - XS No.5 @ RS 19+50

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.075	
Channel Slope:	0.02173	ft/ft
Normal Depth:	3.25	ft
Elevation Range:	731.61 to 752.00 ft	
Discharge:	320.68	ft³/s





Worksheet for Tributary No.1 - XS No.5 @ RS 19+50

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Input Data		£4./£4
Channel Slope:	0.02173	ft/ft
Discharge:	320.68	ft³/s
A		
Options		
Current Roughness Weighted Metho	ImprovedLotters	
Open Channel Weighted Roughnes:	ImprovedLotters	
Closed Channel Weighted Roughne	Hortons	
-		
Results	0.075	
Roughness Coefficient:	0.075	
Water Surface Elevation:	734.86	ft
Elevation Range:	731.61 to 752.00 ft	
Flow Area:	93.10	ft²
Wetted Perimeter:	73.36	ft
Top Width:	70.40	ft

Normal Depth: 3.25

Normai Deptit.	0.20		
Critical Depth:	2.71		ft
Critical Slope:	0.08730		ft/ft
Velocity:	3.44		ft/s
Velocity Head:	0.18	and the second secon	ft
Specific Energy:	3.44		ft
Froude Number:	0.53		
Flow Type:	Subcritical		

ft

Segment Roughness

Start Station	End Station	Roughness Coefficient
(0+00.00, 752.00)	(1+36.47, 733.61)	0.100
(1+36.47, 733.61)	(1+42.98, 733.61)	0.050
(1+42.98, 733.61)	(2+00.00, 736.60)	0.100

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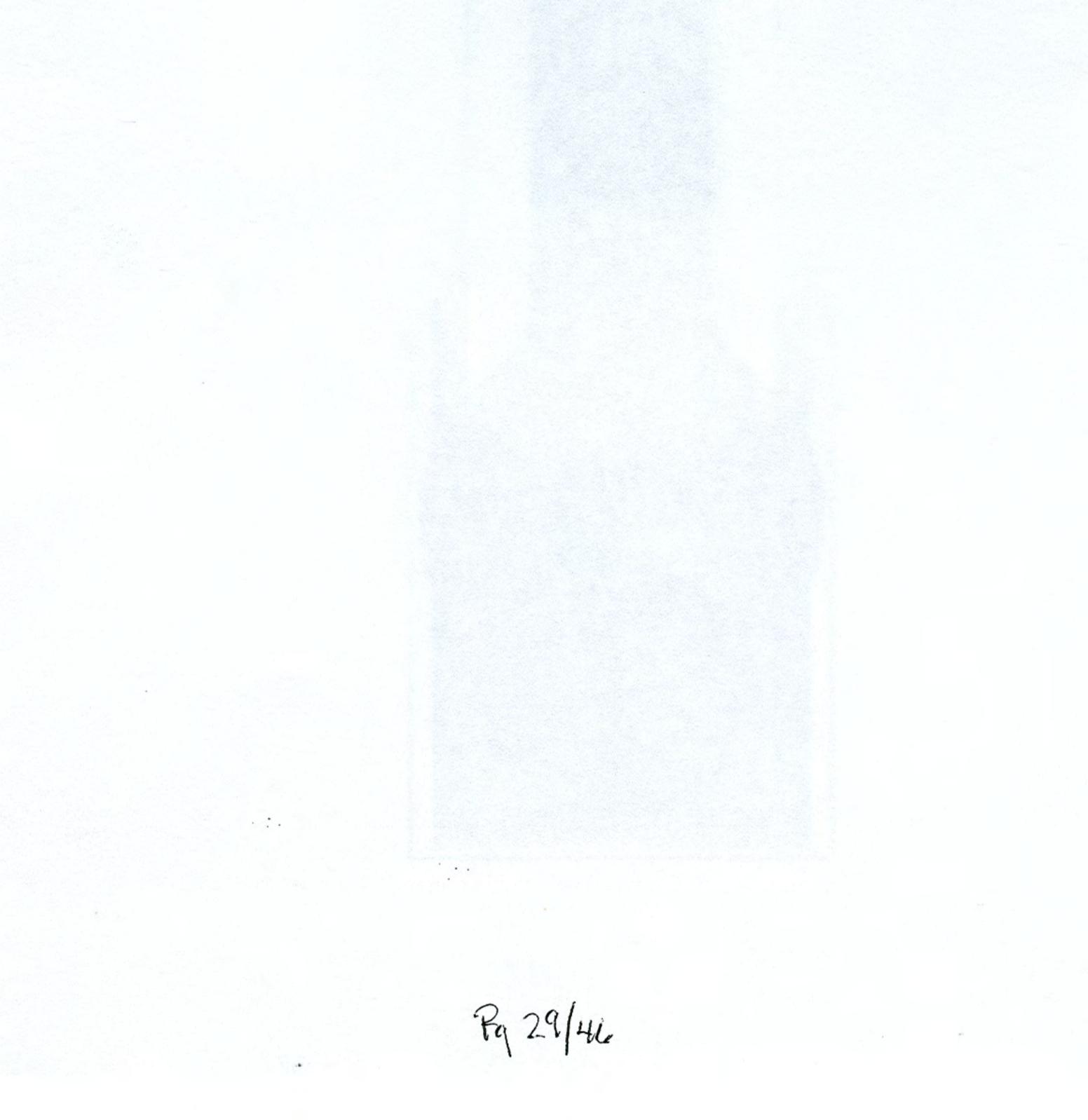
Section Geometry

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Worksheet for Tributary No.1 - XS No.5 @ RS 19+50

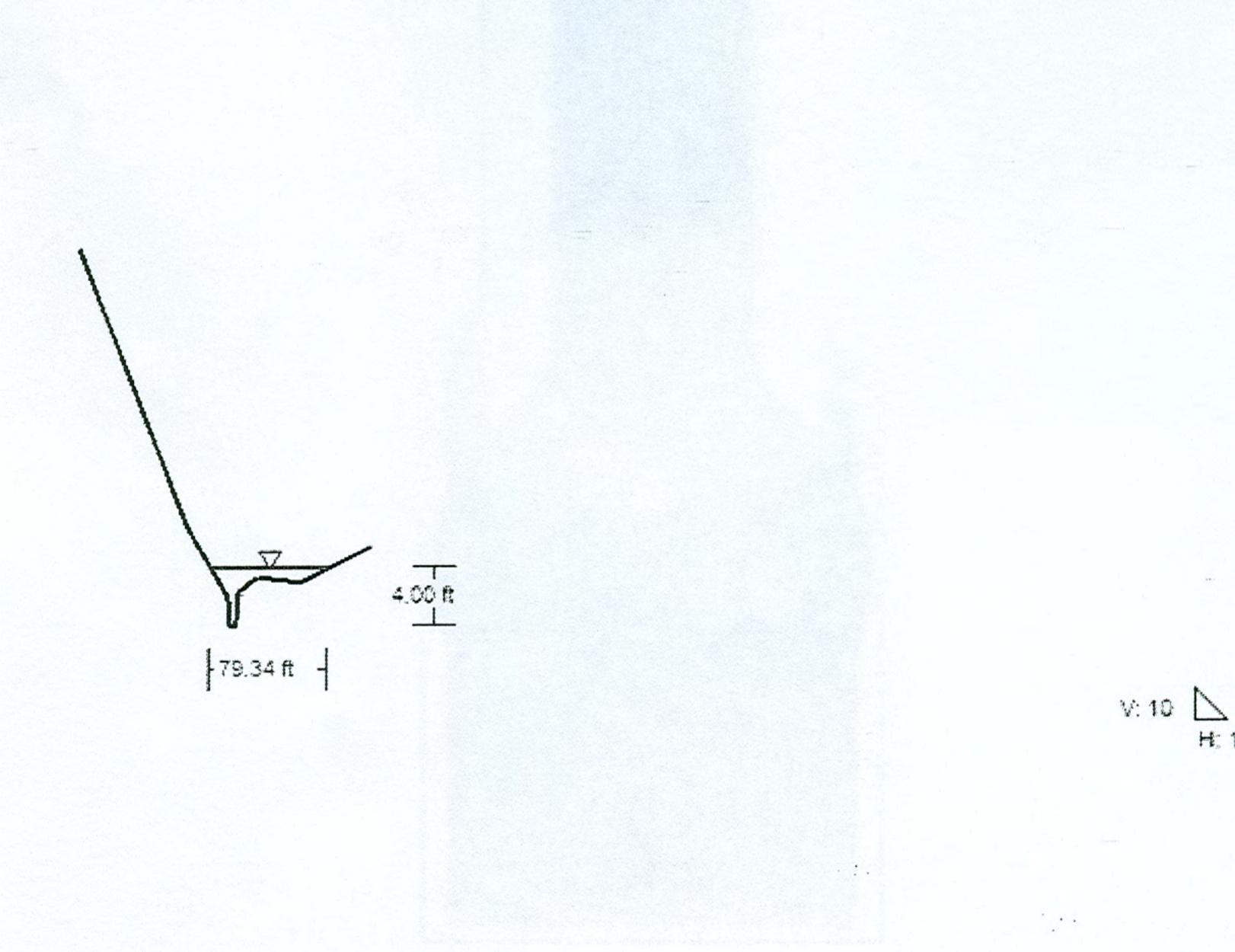
15

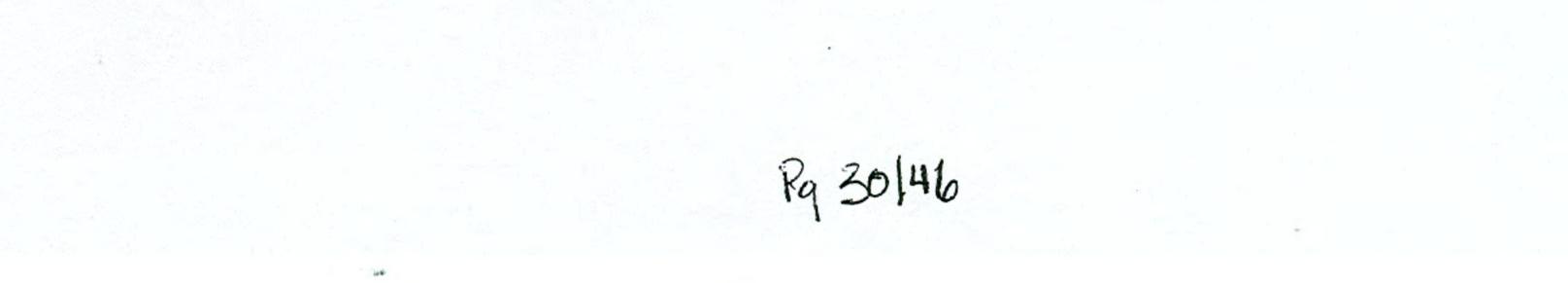
Station	Elevation
0+00.00	752.00
0+50.00	742.10
0+75.00	735.10
1+00.00	733.40
1+25.00	733.30
1+36.47	733.61
1+37.22	731.61
1+42.23	731.61
1+42.98	733.61
1+50.00	735.10
2+00.00	736.60



XS No.4 @ RS 17+75 Cross Section for Tributary No.1 - XS No.4 @ RS 17+75

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.056	
Channel Slope:	0.02173	ft/ft
Normal Depth:	4.00	ft
Elevation Range:	727.73 to 753.40 ft	
Discharge:	320.68	ft³/s





Worksheet for Tributary No.1 - XS No.4 @ RS 17+75

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Input Data		
Channel Slope:	0.02173	ft/ft
Discharge:	320.68	ft³/s
Ontiona		
Options		
Current Roughness Weighted Metho	ImprovedLotters	
Open Channel Weighted Roughnes:	ImprovedLotters	
Closed Channel Weighted Roughne	Hortons	
Deputto		
Results		
Roughness Coefficient:	0.056	
Water Surface Elevation:	731.73	ft
Elevation Range:	727.73 to 753.40 ft	
Flow Area:	82.50	ft²
Wetted Perimeter:	82.24	ft
Top Width:	79.34	ft
Normal Depth:	4.00	ft

ft

ft/ft

ft/s

ft

ft

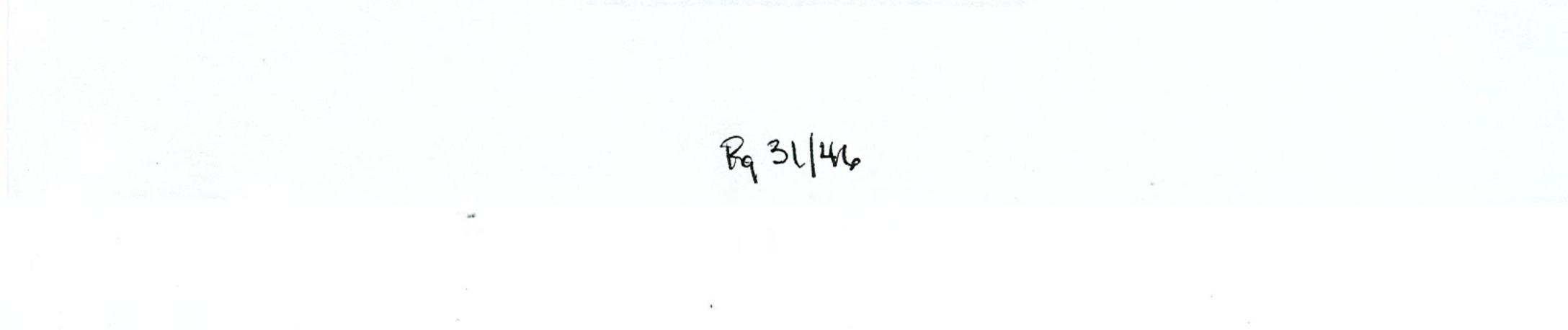
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Normal Depth:	4.00
Critical Depth:	3.72
Critical Slope:	0.05166
Velocity:	3.89
Velocity Head:	0.23
Specific Energy:	4.23
Froude Number:	0.67
Flow Type:	Subcritical

Segment Roughness

End Station	Roughness Coefficient
(1+02.57, 729.77)	0.100
(1+09.74, 730.15)	0.050
(2+00.00, 733.20)	0.100
	(1+02.57, 729.77) (1+09.74, 730.15) (2+00.00,

Section Geometry



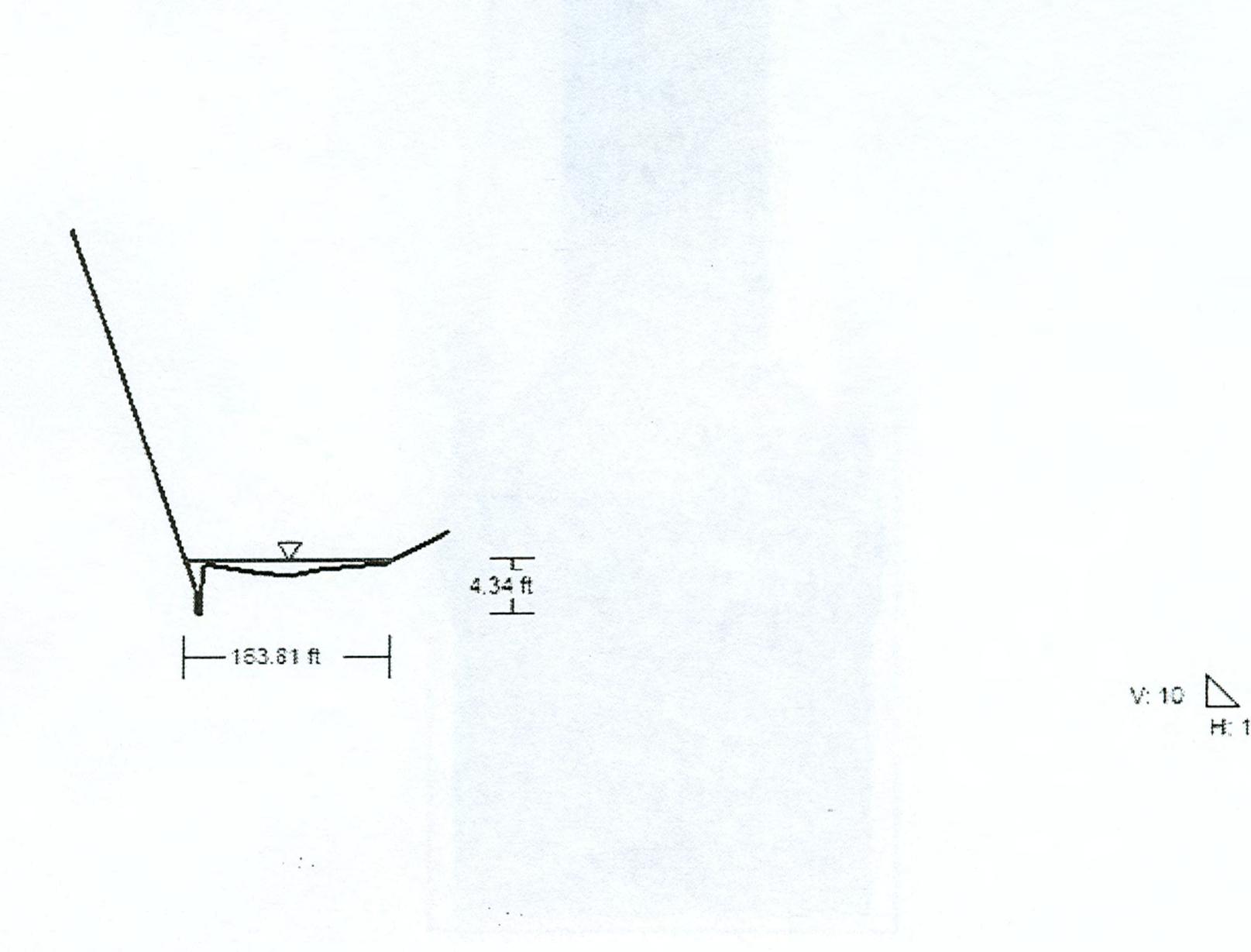
Worksheet for Tributary No.1 - XS No.4 @ RS 17+75

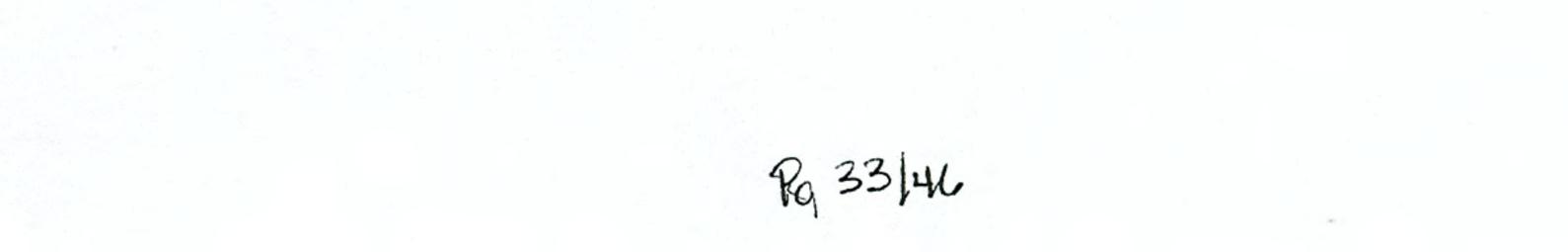
Station	Elevation
0+00.00	753.40
0+75.00	734.50
1+02.57	729.77
1+03.27	727.76
1+08.00	727.73
1+09.74	730.15
1+22.64	731.13
1+51.28	730.77
2+00.00	733.20



XS No.3 @ RS 16+25 Cross Section for Tributary No.1 - XS No.3 @ RS 16+25

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.080	
Channel Slope:	0.02173	ft/ft
Normal Depth:	4.34	 ft
Elevation Range:	724.49 to 754.90 ft	
Discharge:	320.68	ft³/s





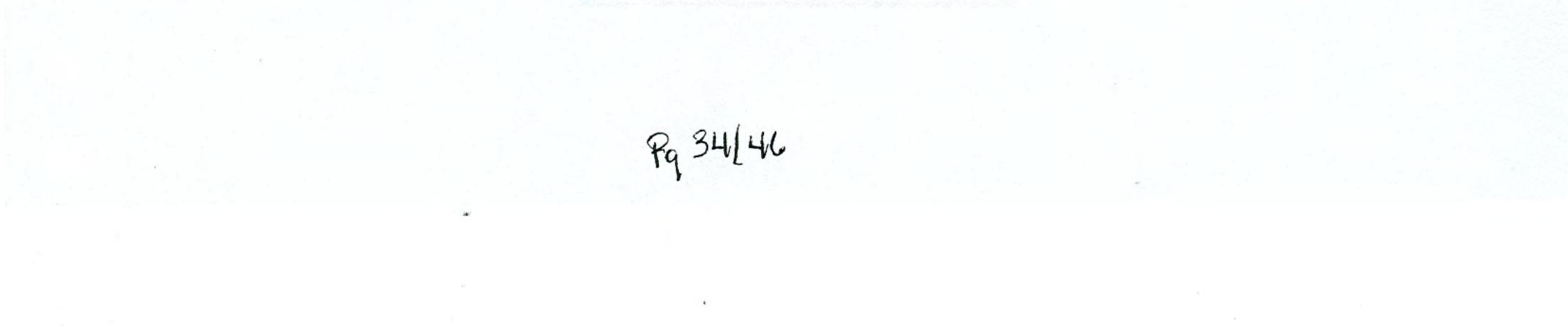
Worksheet for Tributary No.1 - XS No.3 @ RS 16+25

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
	Series and	
Input Data Channel Slope:	0.02173	
Channel Slope:	320.68	
Discharge:	320.00	
Options		
Current Roughness Weighted Metho	ImprovedLotters	
Open Channel Weighted Roughnes:	ImprovedLotters	
Closed Channel Weighted Roughne	Hortons	
Results	0.080	
Roughness Coefficient:	0.080	
Water Surface Elevation:	728.83	
Elevation Range:	724.49 to 754.90 f	
Flow Area:	135.72	
Wetted Perimeter:	167.95	
Top Width:	163.81	
Normal Depth:	4.34	
Critical Depth:	3.97	
Critical Slope:	0.12044	
Velocity:	2.36	
Velocity Head:	0.09	
Specific Energy:	4.42	
Froude Number:	0.46	
Flow Type:	Subcritical	

Segment Roughness

Start Station	End Station	Roughness Coefficient
(0+00.00, 754.90)	(1+00.20, 726.51)	0.100
(1+00.20, 726.51)	(1+03.26, 725.69)	0.050
(1+03.26, 725.69)	(3+00.00, 731.08)	0.100

Section Geometry



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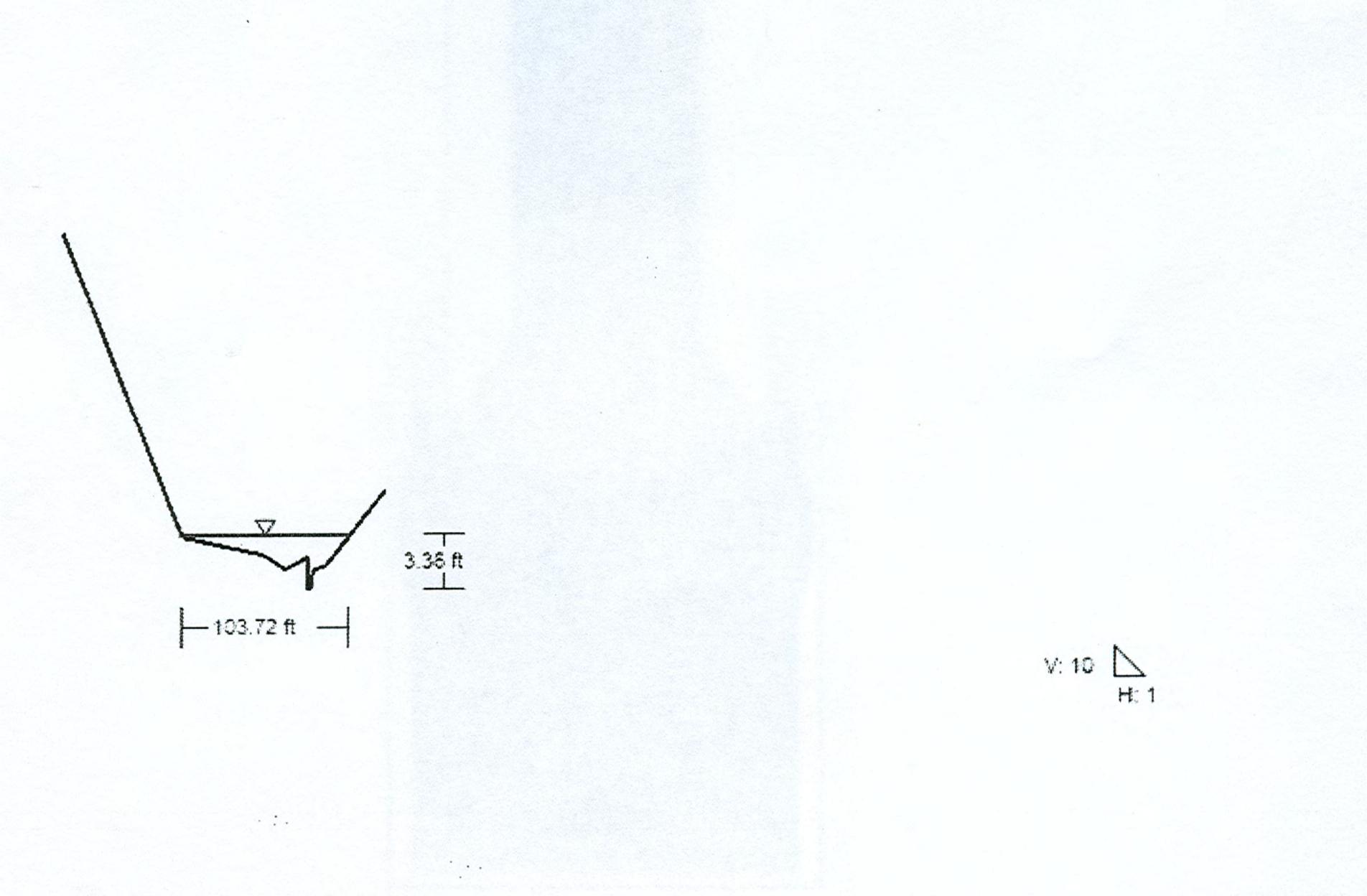
Worksheet for Tributary No.1 - XS No.3 @ RS 16+25

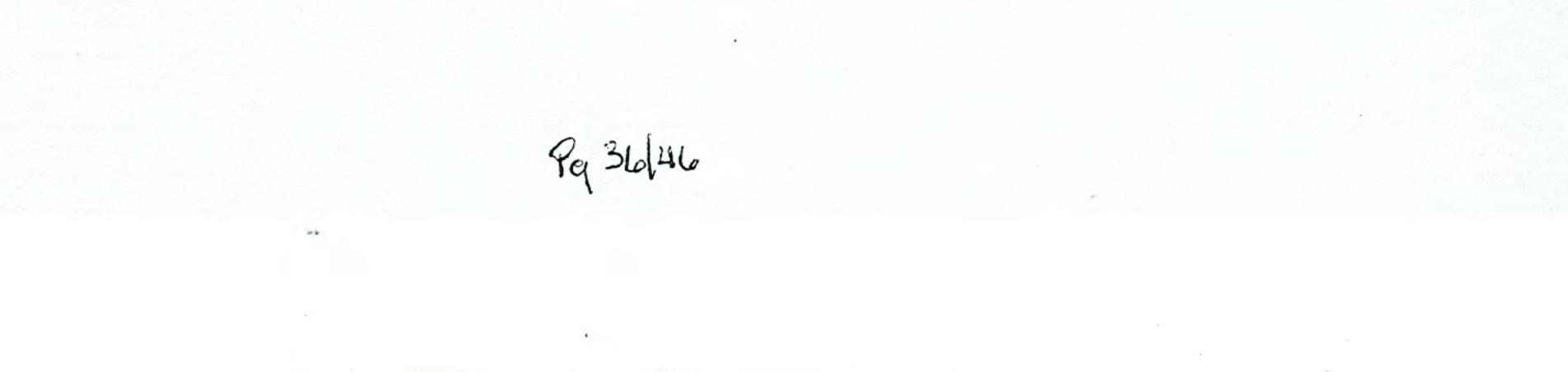
Station	Elevation
0+00.00	754.90
0+75.00	733.60
1+00.20	726.51
1+00.46	724.51
1+03.01	724.49
1+03.26	725.69
1+06.51	728.52
1+50.00	727.80
1+75.00	727.60
2+00.00	728.10
2+50.00	728.53
3+00.00	731.08



XS No.2 @ RS 13+75 Cross Section for Tributary No.1 - XS No.2 @ RS 13+75

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.089	
Channel Slope:	0.02173	ft/ft
Normal Depth:	3.36	ft
Elevation Range:	719.07 to 741.10 ft	
Discharge:	320.68	ft³/s





Worksheet for Tributary No.1 - XS No.2 @ RS 13+75

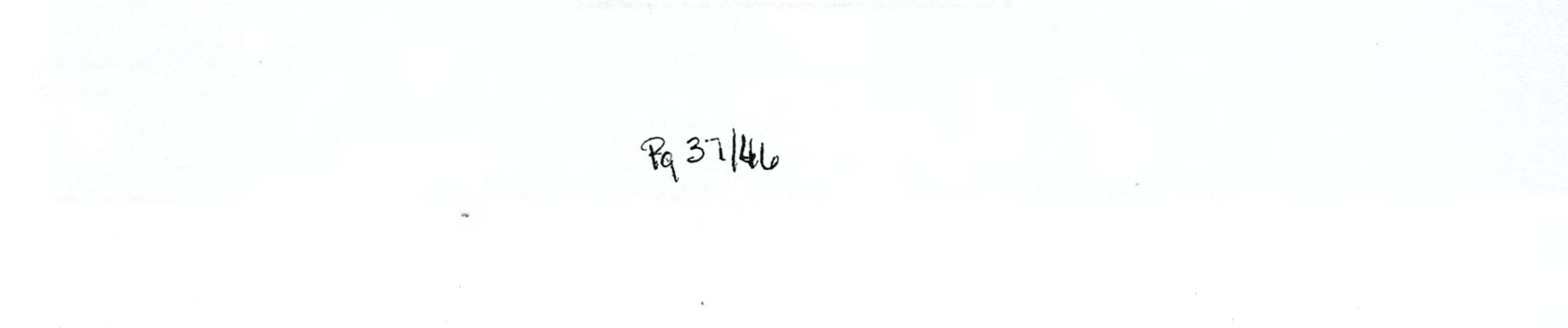
Project Description			
Flow Element:	Irregular Section		
Friction Method:	Manning Formula		
Solve For:	Normal Depth		
Input Data			
Channel Slope:	0.02173		ft/ft
Discharge:	320.68		ft³/s
Options			
Current Roughness Weighted Metho	ImprovedLotters		
Open Channel Weighted Roughnes:	ImprovedLotters		
Closed Channel Weighted Roughne	Hortons		
Results			
Roughness Coefficient:	0.089		
Water Surface Elevation:	722.43		ft
Elevation Range:	719.07 to 741.10 ft		
Flow Area:	120.36		ft²
Wetted Perimeter:	106.65		ft
Top Width:	103.72		ft
Normal Depth:	3.36		ft
Critical Depth:	2.72		ft
Critical Slope:	0.12961		ft/ft
Velocity:	2.66		ft/s
Velocity Head:	0.11	a second and a second	ft
Specific Energy:	3.47		ft
Froude Number:	0.44		
Flow Type:	Subcritical		

Segment Roughness

32 S

Start Station	End Station	Roughness Coefficient
(0+00.00, 741.10)	(1+52.37, 721.07)	0.100
(1+52.37, 721.07)	(1+55.45, 720.28)	0.050
(1+55.45, 720.28)	(2+00.00, 725.20)	0.100

Section Geometry



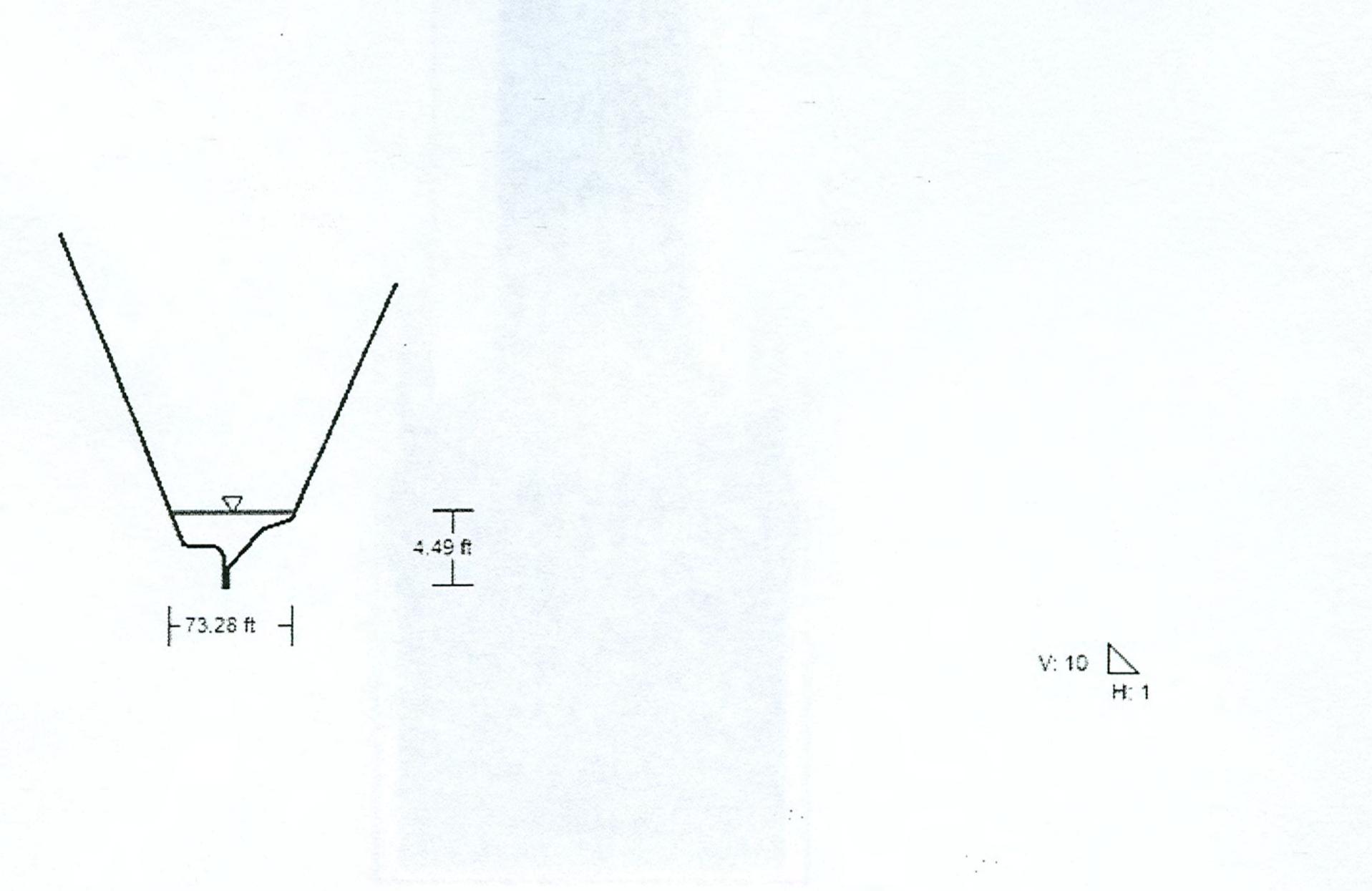
Worksheet for Tributary No.1 - XS No.2 @ RS 13+75

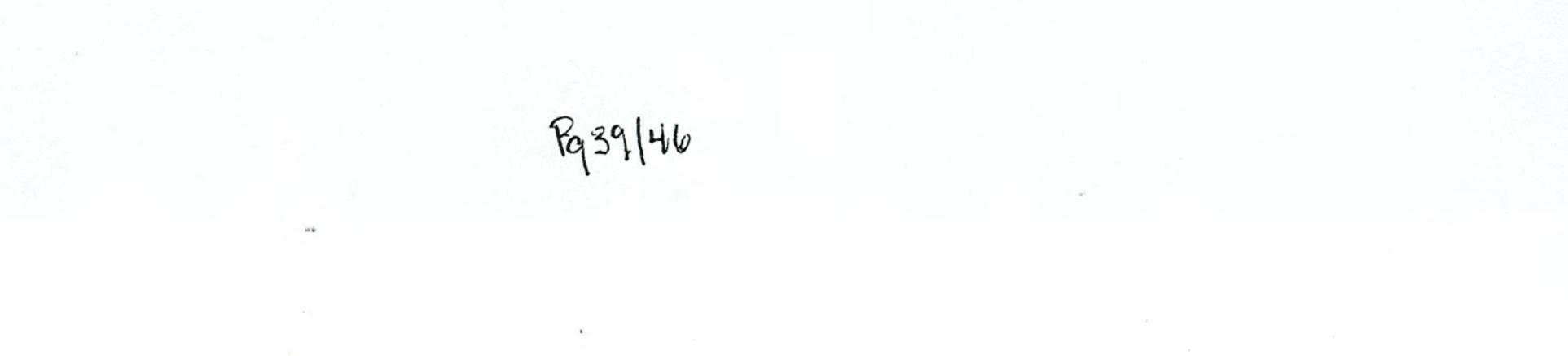
Station	Elevation	
0+00.00	741.10	
0+75.00	722.30	
1+25.00	721.20	
1+38.54	720.32	
1+52.37	721.07	
1+52.63	719.07	
1+55.19	719.08	
1+55.45	720.28	
1+63.52	720.56	
2+00.00	725.20	



XS No.1 @ RS 12+50 Cross Section for Tributary No.1 - XS No.1 @ RS 12+50

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	
Section Data		
Roughness Coefficient:	0.088	
Channel Slope:	0.01325	ft/ft
Normal Depth:	4.49	ft
Elevation Range:	716.90 to 738.00 ft	
Discharge:	320.68	ft³/s





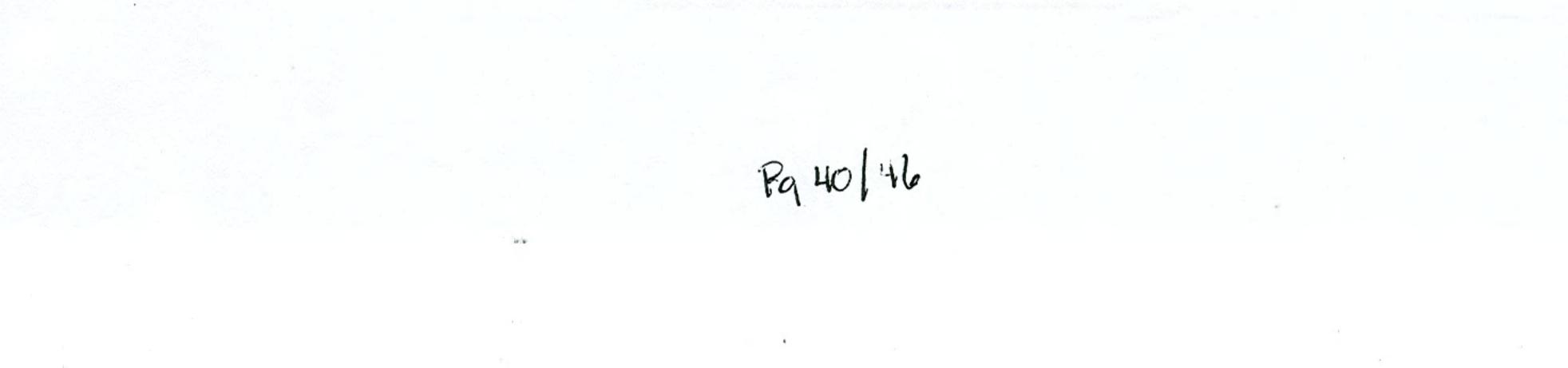
Worksheet for Tributary No.1 - XS No.1 @ RS 12+50

 Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Normal Depth	1
Input Data	0.01225	ft/ft
Channel Slope:	0.01325	ft³/s
Discharge:	320.68	1175
Options		
Current Roughness Weighted Metho	ImprovedLotters	
Open Channel Weighted Roughnes:	ImprovedLotters	
Closed Channel Weighted Roughne	Hortons	
Results		
Roughness Coefficient:	0.088	
Water Surface Elevation:	721.39	ft
Elevation Range:	716.90 to 738.00 ft	
Flow Area:	121.58	ft²
Wetted Perimeter:	76.52	ft
Top Width:	73.28	ft
Normal Depth:	4.49	ft
Critical Depth:	3.43	ft
Critical Slope:	0.11924	ft/ft
Velocity:	2.64	ft/s
Velocity Head:	0.11	ft
Specific Energy:	4.60	ft
Froude Number:	0.36	
Flow Type:	Subcritical	

Segment Roughness

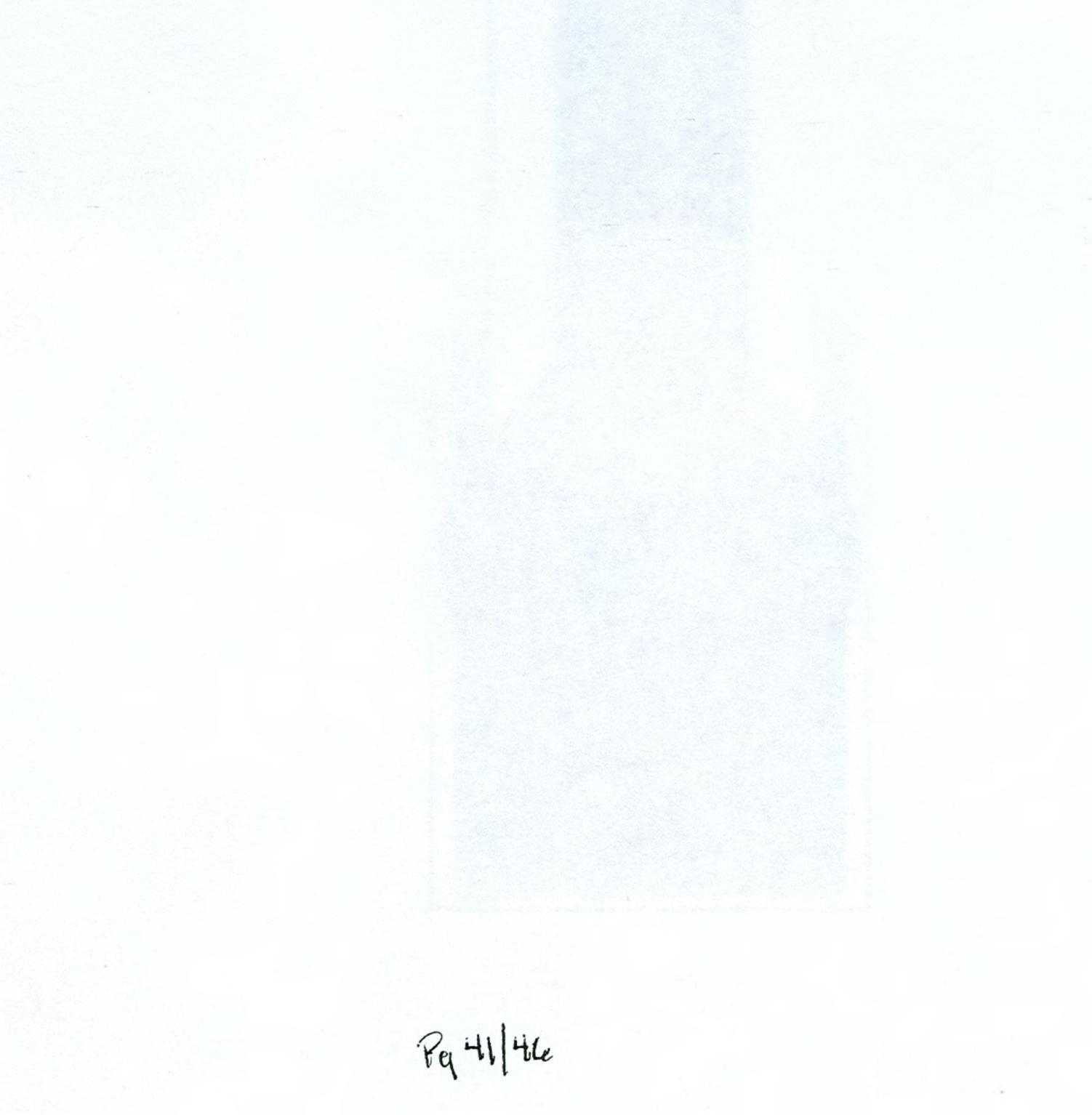
Start Station	End Station	Roughness Coefficient
(0+00.00, 738.00)	(0+98.62, 718.90)	0.100
(0+98.62, 718.90)	(1+01.37, 718.10)	0.050
(1+01.37, 718.10)	(2+00.00, 734.90)	0.100

Section Geometry

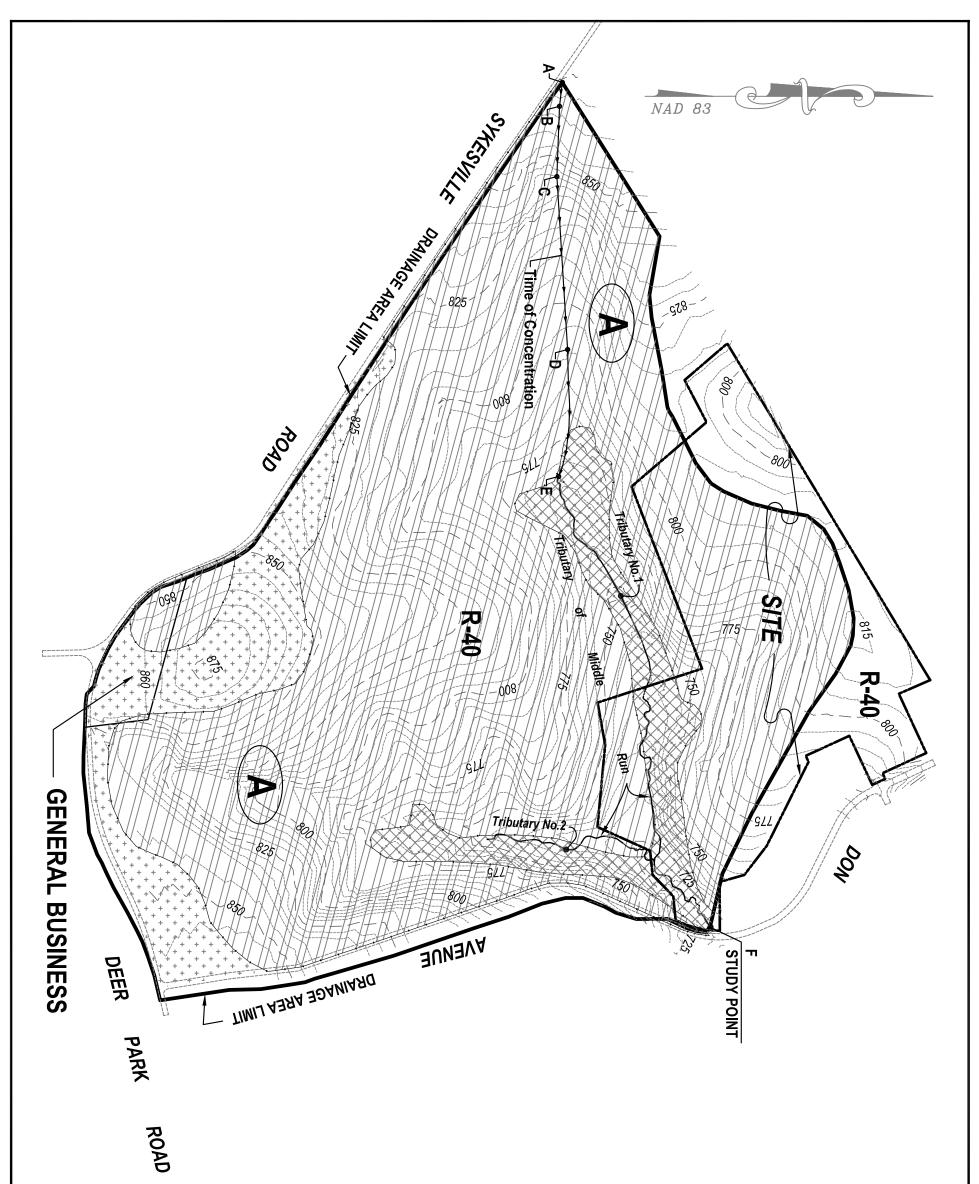


Worksheet for Tributary No.1 - XS No.1 @ RS 12+50

Station	Elevation
0+00.00	738.00
0+75.00	719.50
0+95.67	719.36
0+98.62	718.90
0+98.85	716.90
1+01.15	716.90
1+01.37	718.10
1+21.51	720.43
1+38.87	721.00
2+00.00	734.90

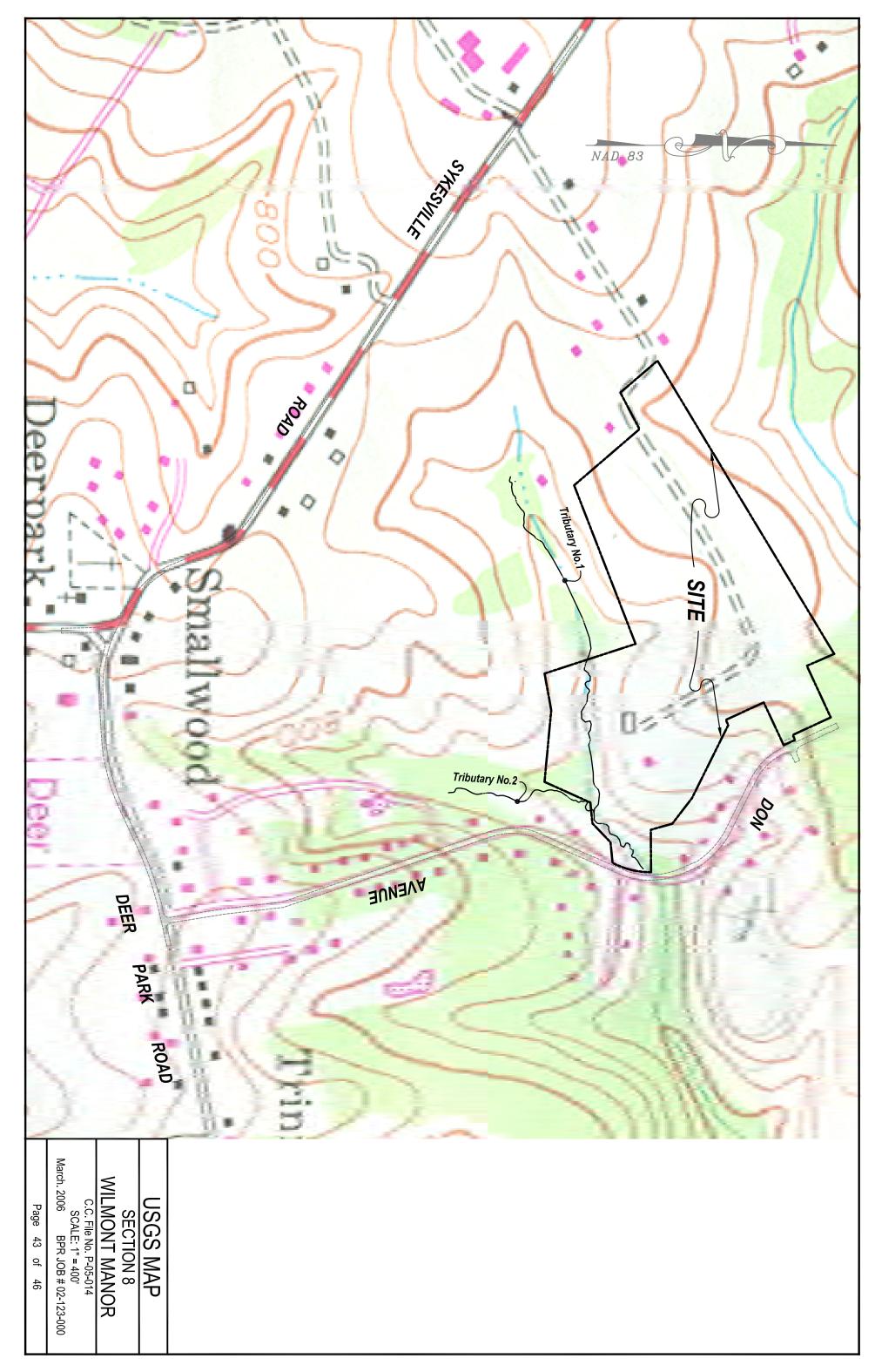


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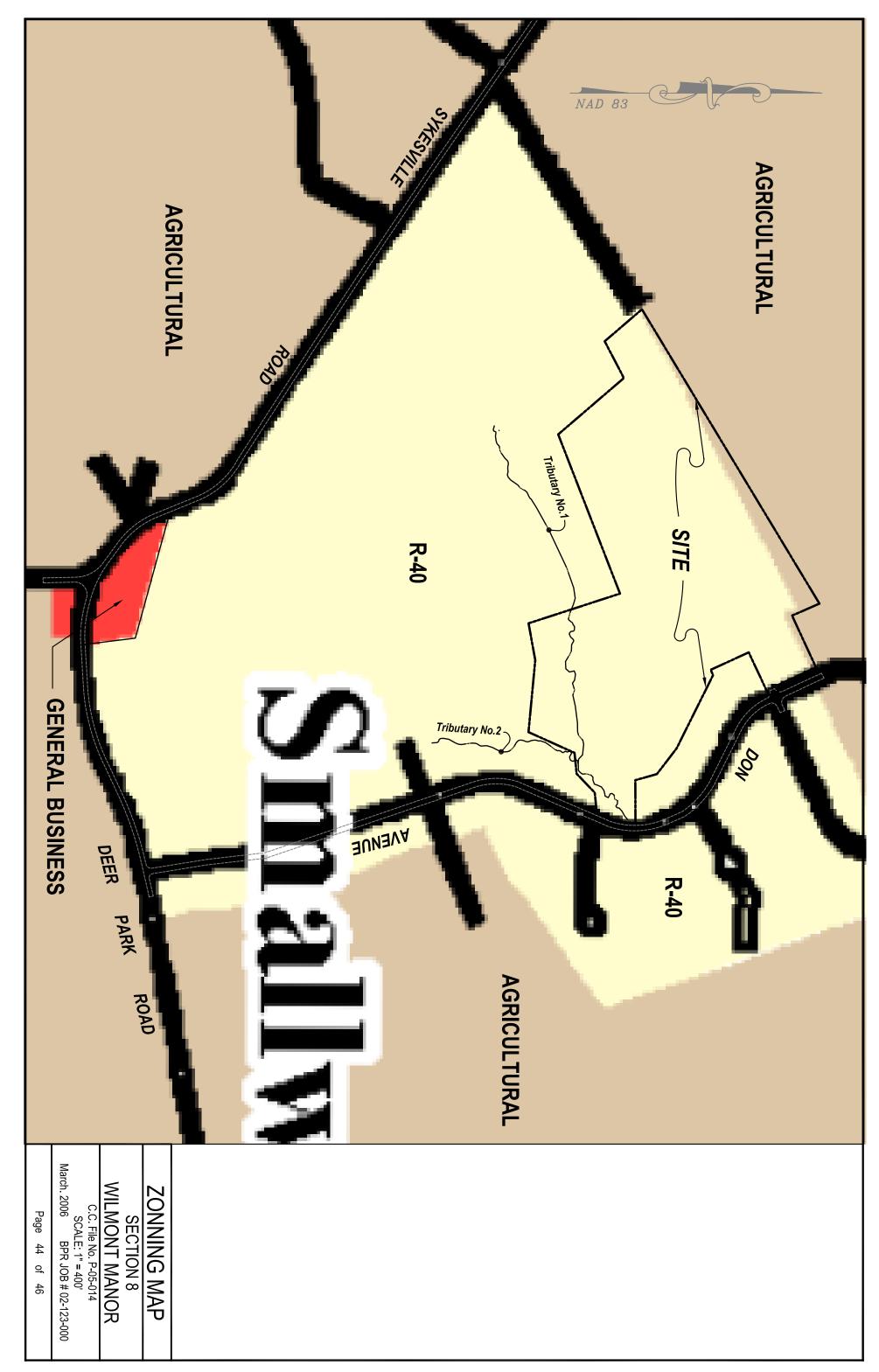


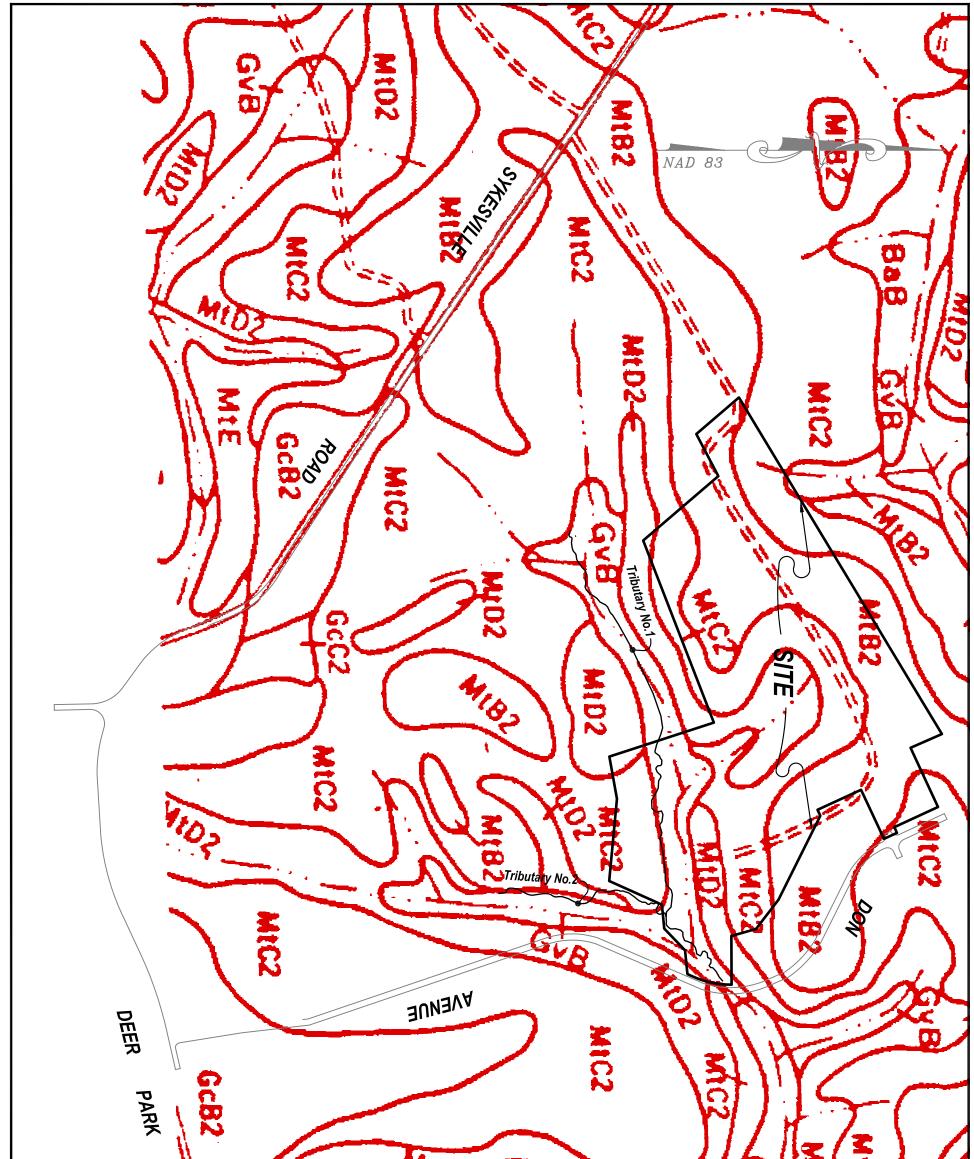
Page 42 of 46	SPR 1	CTION 8	DRAINAGE AREA MA	DRAINAGE AREA: DA 'A' = 155.2 Acres	3. SDC SOILS MAP - MAP No.36	2. C.C. ZONNING MAP - GENERALIZED ZONNING MAP OF CARROLL COUNTY	 MAPPING SOURCES: 1. TOPOGRAPHY MAPPING: 5' Contour Interval A. Field run survey by BPR, Inc. for WILMONT MANOR Section 8 - CC File No. P-05-014 B. Aerial topography, Flown for VICTORIA FARMS - CC File No. P-02-011 C. Aerial topography, Flown for FORSTER PROPERTY - BPR Job No. 04-160-000 D. USGS Quad Map - Westminster 	"A" SOILS "C" SOILS
	00	:	₽			JNTY	0 014	

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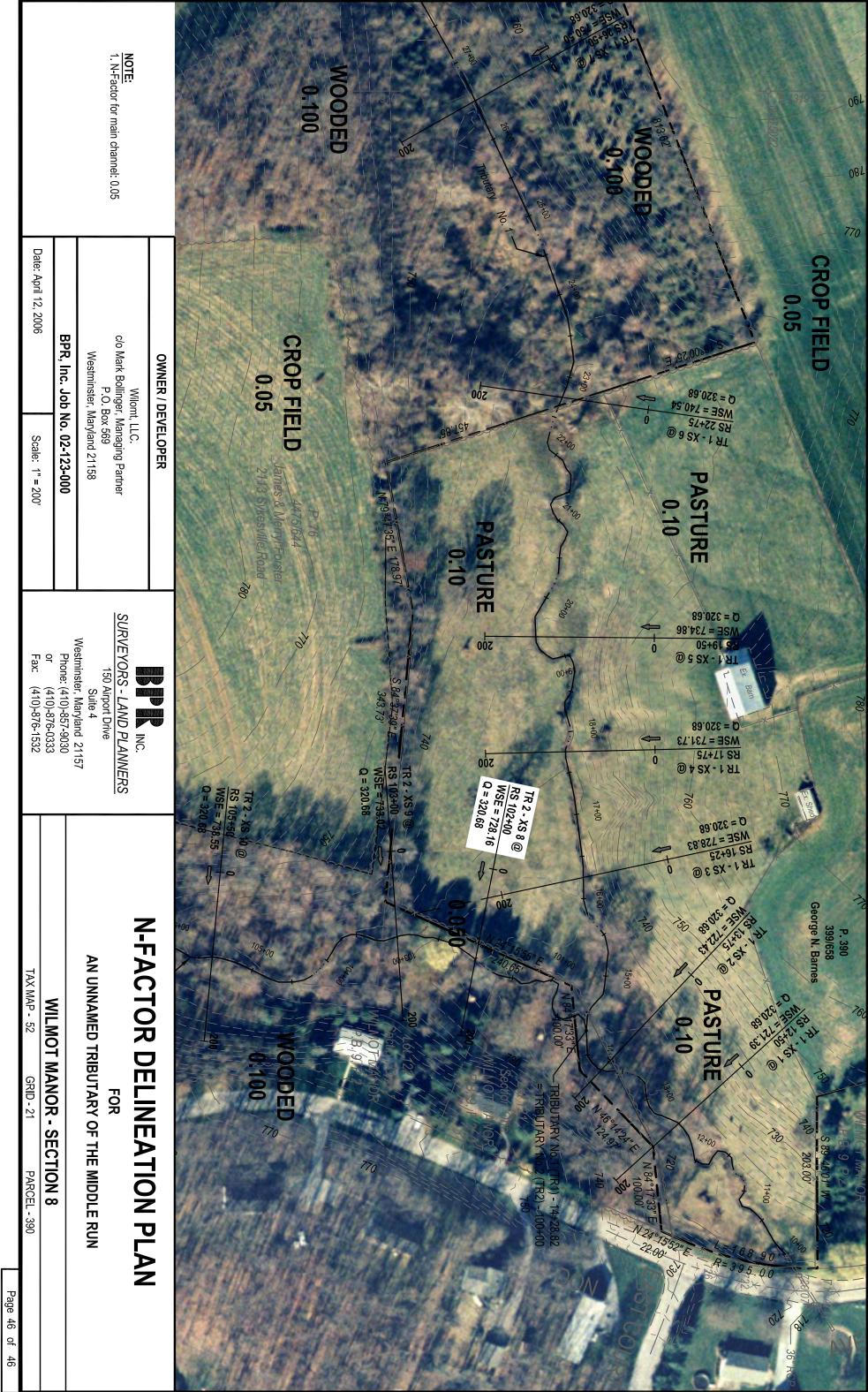


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ROAD	
SOILS MAP SECTION 8 WILMONT MANOR C.C. File No. P-05-014 SCALE: 1" = 400' March. 2006 BPR JOB # 02-123-000 Page 45 of 46	



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